

LONDON LOCATION

1599 Adelaide St. N., Units 301 & 203 London, ON N5X 4E8 P: 519-471-6667

KITCHENER LOCATION

1415 Huron Rd., Unit 225 Kitchener, ON N2R 0L3 P: 519-725-8093

www.sbmltd.ca

sbm@sbmltd.ca

18 March 2021 SBM-21-0716

The Municipality of Middlesex Centre 10227 Ilderton Road, RR#2 Ilderton, ON, NOM 2A0

Attn: Jake DeRidder, C. Tech.

Development Review Coordinator

Re: Brock Development Group

Proposed 19 Residential Lots Development

Medway Road, Ballymote, Ontario

1. INTRODUCTION

This Site Servicing Memorandum (Memo) has been prepared by Strik, Baldinelli, Moniz Ltd (SBM) to address the site servicing requirement for the proposed development of 19 residential lots at Highbury Avenue North and Medway Road, Ballymote, Ontario. The site is bordered by general agricultural zones to its east and south, Hamlet Residential First Density zones across the Highbury Avenue North Right-of-Way (ROW) to the west and the Medway Road ROW to the north. This Memo is to determine the adequacy of the existing municipal services in support of Zoning By-law Amendment (ZBA) applications for the proposed lot severance.

2. SANITARY SERVICING CONSIDERATIONS

As per the Ballymote Waterworks Highbury Avenue as-constructed drawings provided by the Municipality, Project No. 92037 sheets 1, 2 6, and 7, all dated April 15, 1994, provided in Appendix A, the proposed development is not tributary to a sanitary sewer. There are no existing sanitary sewers on Highbury Avenue North ROW or Medway Road ROW and therefore, on-site septic systems are proposed.

The dimensions of the on-site septic system and contingency area are obtained from the Soil Report for Wastewater Servicing by BOS Engineering & Environmental Services Inc., dated January 4, 2021, provided in Appendix B. The system is schematically shown on Engineering Plans sheets 2A and 2B provided separately.

3. STORMWATER MANAGEMENT

The is no storm outlet for the proposed development to handle storm flows, therefore a soak-away pit is proposed on each property.

3.1 Pre-Development to Post-Development Conditions

Pre-development conditions were obtained from the topographical plan of survey by Callon Dietz Incorporated, File No. 20-23779 A, dated February 17, 2021, provided in Appendix C. The pre-development peak outflows

during different storm events are to be calculated as the design flow using the total area, runoff coefficients and average rainfall intensity as per Section 4.8 of Middlesex Centre Infrastructure Design Standards (MCIDS). Similarly, the post-development peak outflows are calculated and the differences between the predevelopment to post-development are used to calculate the required storage and size of the soak-away pit.

The lot sizes are similar but varies, and therefore five different sizes of soakaway pit are designed for appropriate lot sizes. Pre to post development comparison is made in the Storm Water Management (SWM) calculations attached in Appendix C and the soak-away pit is designed to infiltrate and store any additional flows to balance the total outflow up to 100-year storm events (major event). Actual post-development C-values were calculated to be less than 0.40, but conservatively C-value of 0.50 was used in accordance with Section 4.8.3 of Municipality of Middlesex Centre Infrastructure Design Standards. Soak-away pits with dimensions 3.0 m wide by varying length at 1.0 m depth constructed with 19 mm clear stone will provide sufficient storages (void ratio of 0.35) required during 100-year storm events. It is noted that the soil condition on site is obtained from the BOS Engineering Soil Report mentioned above at 50 min/cm infiltration rate. This is also assuming that in pre-development condition, the ditches in the Highbury Avenue North ROW and Medway Road ROW is designed to take some outflows from the lots calculated with the runoff coefficient of 0.2. See below Table 1 for summary of the five different soakaway pit sizes for different lots.

Lot #	Dimension	2-yr Required	100-yr Required	Available Storage
		Storage (m³)	Storage (m³)	(m³)
1, 2 & 3	3.0m x 26.5m x 1.0m	16.05	27.81	27.83
4	3.0m x 25.7m x 1.0m	15.54	26.92	26.99
5 to 14 & 17 to 19	3.0m x 22.1m x 1.0m	13.37	23.17	23.21
15	3.0m x 23.1m x 1.0m	13.96	24.20	24.26
16	4.0m x 21.0m x 1.0m	12.67	21.96	22.05

Table 1: Summary of Soakaway Pit Design

Lots 4, 5 and 16 are at the lowest elevations from each of the three rows, and there are surface ponding with depths of 18 mm, 60 mm, and 19 mm expected during storm events respectively. Draw down times of all surface ponding have been calculated to be approximately 11.0 hrs, 1.0 hr and 12.9 hrs respectively.

As the existing ditch on Medway Road ROW has an 800 mm diameter storm outlet, proposed driveway culverts are designed to match existing ditch flow capacity and the slopes of the existing ditches are to remain. A swale is proposed in between the lots and the field to direct all stormwater flow to the existing municipal ditch on Medway Road ROW generally matching the pre-development flow path.

4. WATER SERVICING CONSIDERATIONS

4.1 General Consideration

Water service connections are to be sized as per Section 5.9.1 of the MCIDS. Water services shall be a minimum of 25 mm (1") internal diameter, equipped with approved corporation stop and curb stop with a curb box, and shall be installed as per Figure 5.10 Standard Installation of <50mm Water Service Connection and Layout Detail. The corporation stop shall be installed at the watermain, and a curb stop shall be installed 0.3 m from, and on the street side of, the property line as per Figure 5.11.

Based on the above criteria, a 25 mm (1") PEX water service connection with tracer wire full length is proposed, connecting to municipal water main with approved service saddles and main stops as per Figure 5.10. As per table A-7.6.3.1 of the Ontario Building Code 2012 (OBC), 60 m length of this water service connection can serve 57 fixture units, which is a conservative threshold for a single-family residential building. For reference, according to table 7.4.9.3 of OBC, a single-family dwelling with 3 bathroom groups, a clothes washer, a dishwasher, a floor drain, a laundry tray, and a sink will have 25.5 fixture units.

4.2 Fire-fighting Consideration

As per Section 5.8.1 of the MCIDS, the location of hydrants is subject to the requirements and approval of the Municipal Fire Department in accordance with the Ontario Building Code. As a general guide, hydrants must be located not more than 170 m apart along the length of the watermain and should be located at intersections where possible. There is an existing Municipal fire hydrant across the Medway Road available for Lots 1, 2, 3 and 4. There is also an existing Municipal fire hydrant at southeast side of Medway Road and Highbury Avenue North intersection and at the front of Lot 15. Because the distance between the hydrants appears greater than 170 m on Highbury Avenue North, the development is to follow the local rural requirements and guidelines as per the municipality and/or fire department.

5. LIMITATIONS

This memorandum was prepared by SBM Ltd. for the Municipality of Middlesex Centre and Brock Development Group. Use of this memorandum by any third party, or any reliance upon its findings, is solely the responsibility of that party. SBM Ltd. accepts no responsibility for damages, if any, suffered by a third party as a result of decisions made or actions undertaken as a result of this memorandum. Third party use of this memorandum, without the express written consent of the Consultant, denies any claims, whether in contract, tort, and/or any other cause of action in law, against the Consultant.

All findings and conclusions presented in this memorandum are based on site conditions as they appeared during the period of the investigation. This memorandum is not intended to be exhaustive in scope, or to imply a risk-free facility. It should be recognized that the passage of time may alter the designs, opinions, conclusions, and/or recommendations provided herein.

The design was limited to the documents referenced herein and on the SBM drawings provided separately. SBM Ltd. accepts no responsibility for the accuracy of the information provided by others. All designs, opinions, conclusions, and/or recommendations presented in this memorandum are based on the information available at the time of the review. This document is deemed to be the intellectual property of SBM Ltd. in accordance with Canadian copyright law.

6. CLOSURE

We trust this memorandum meets your satisfaction and current needs. Should you have any questions or require additional information, please do not hesitate to contact the undersigned.

Respectfully submitted,

Strik, Baldinelli, Moniz Ltd.

Planning • Civil • Structural • Mechanical • Electrical

Nelson Guiot, P.Eng.

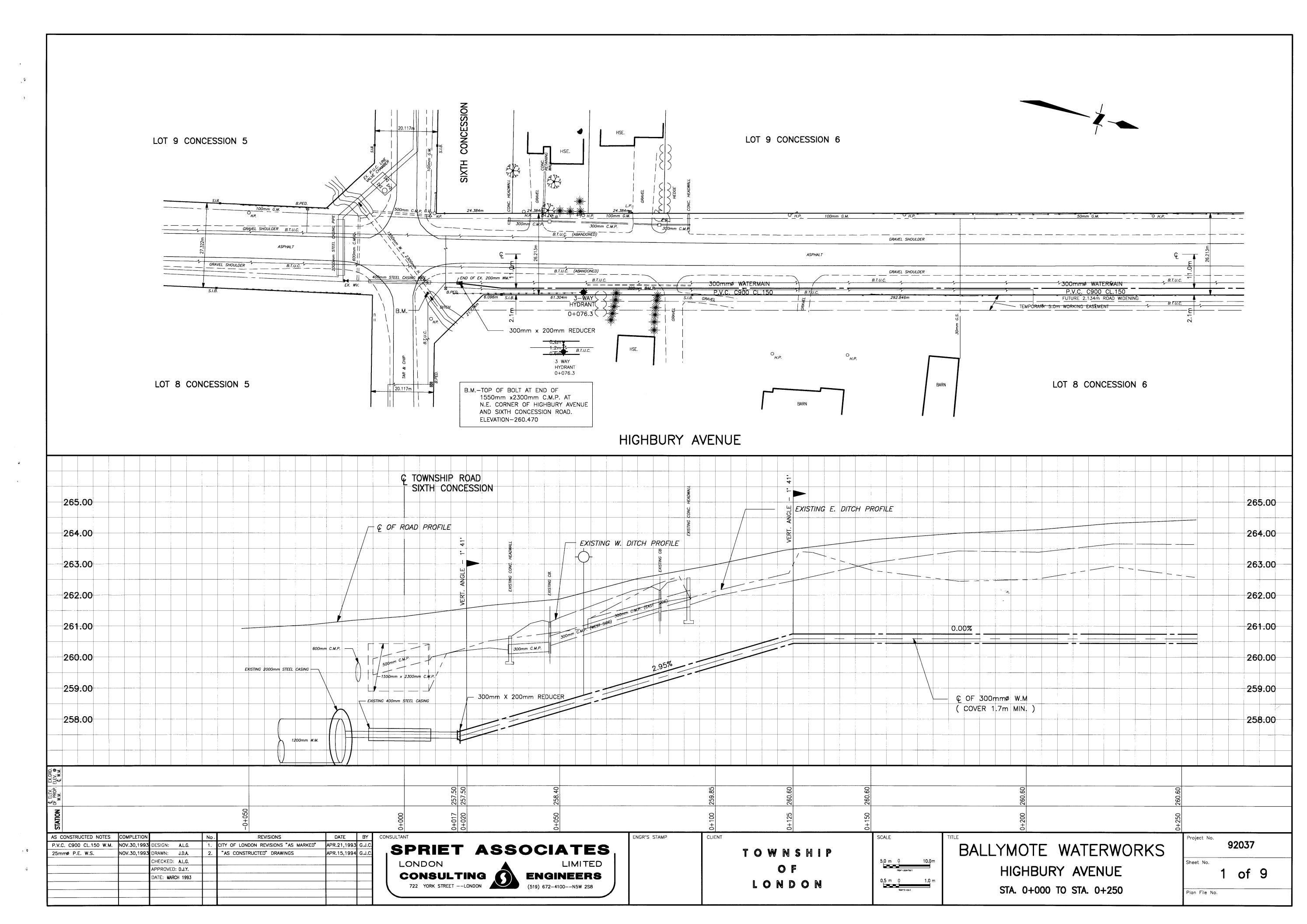
Engineer IV, Civil Department Manager

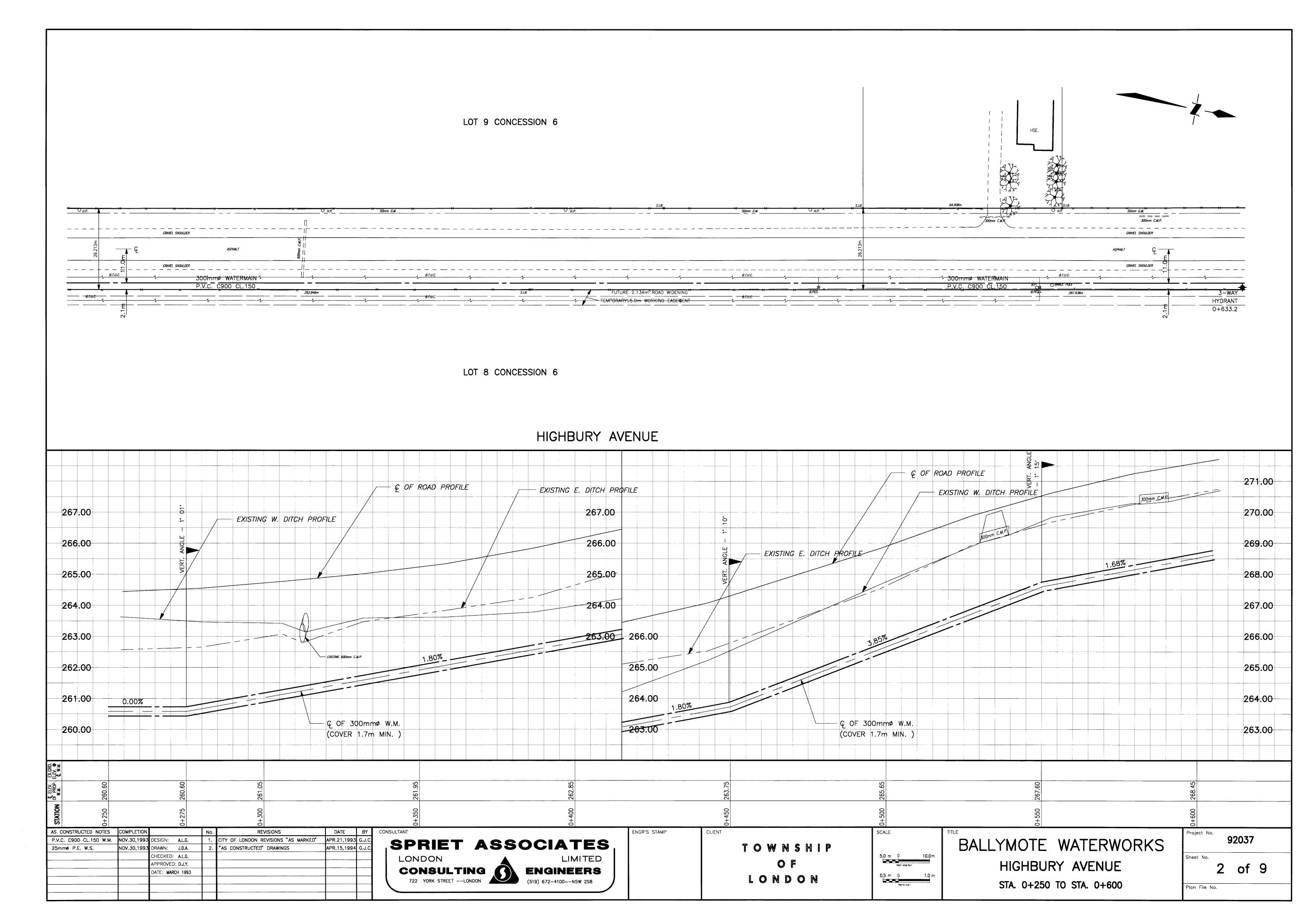
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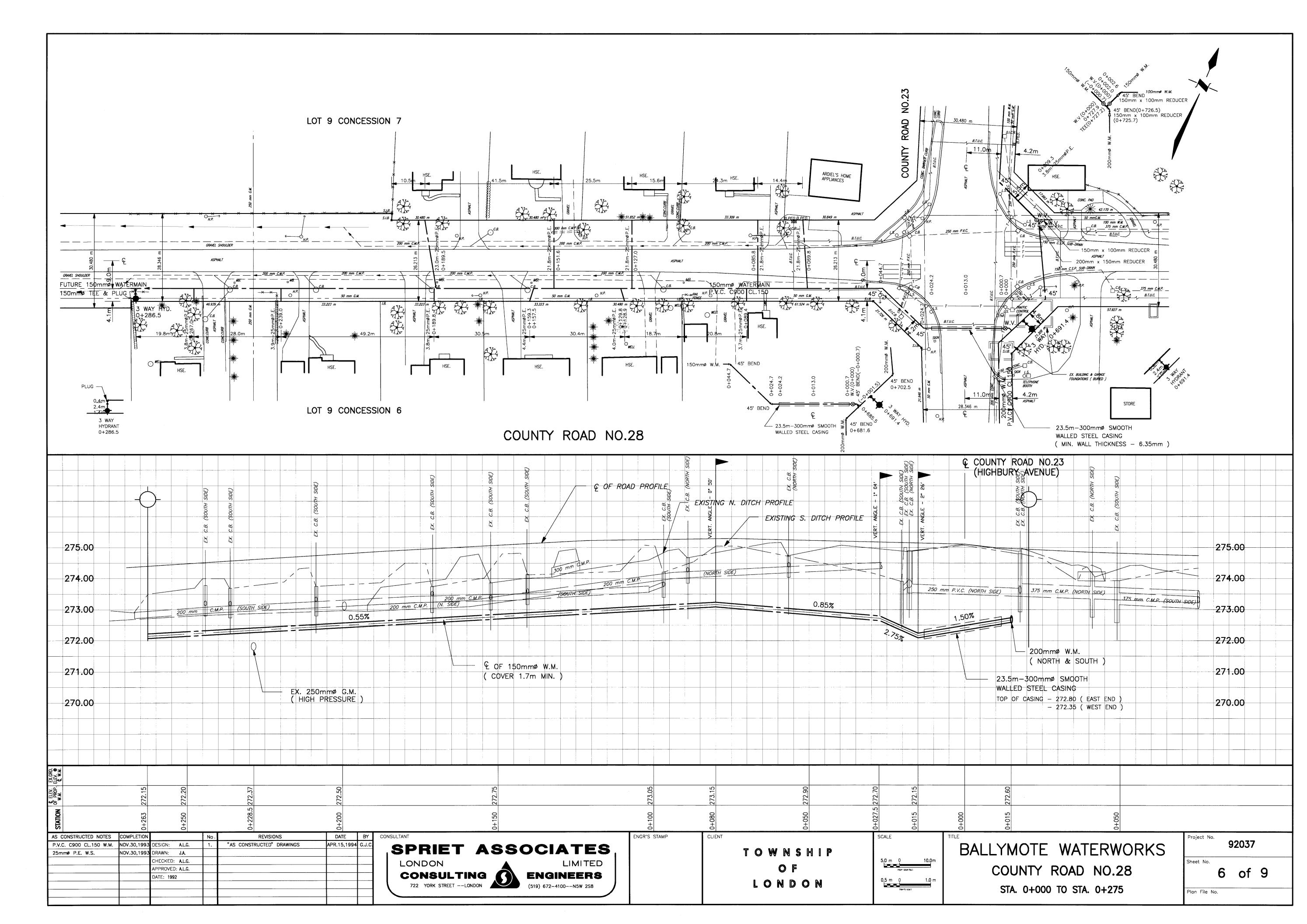
Juduk Lee, E.I.T Civil Engineer In Training I

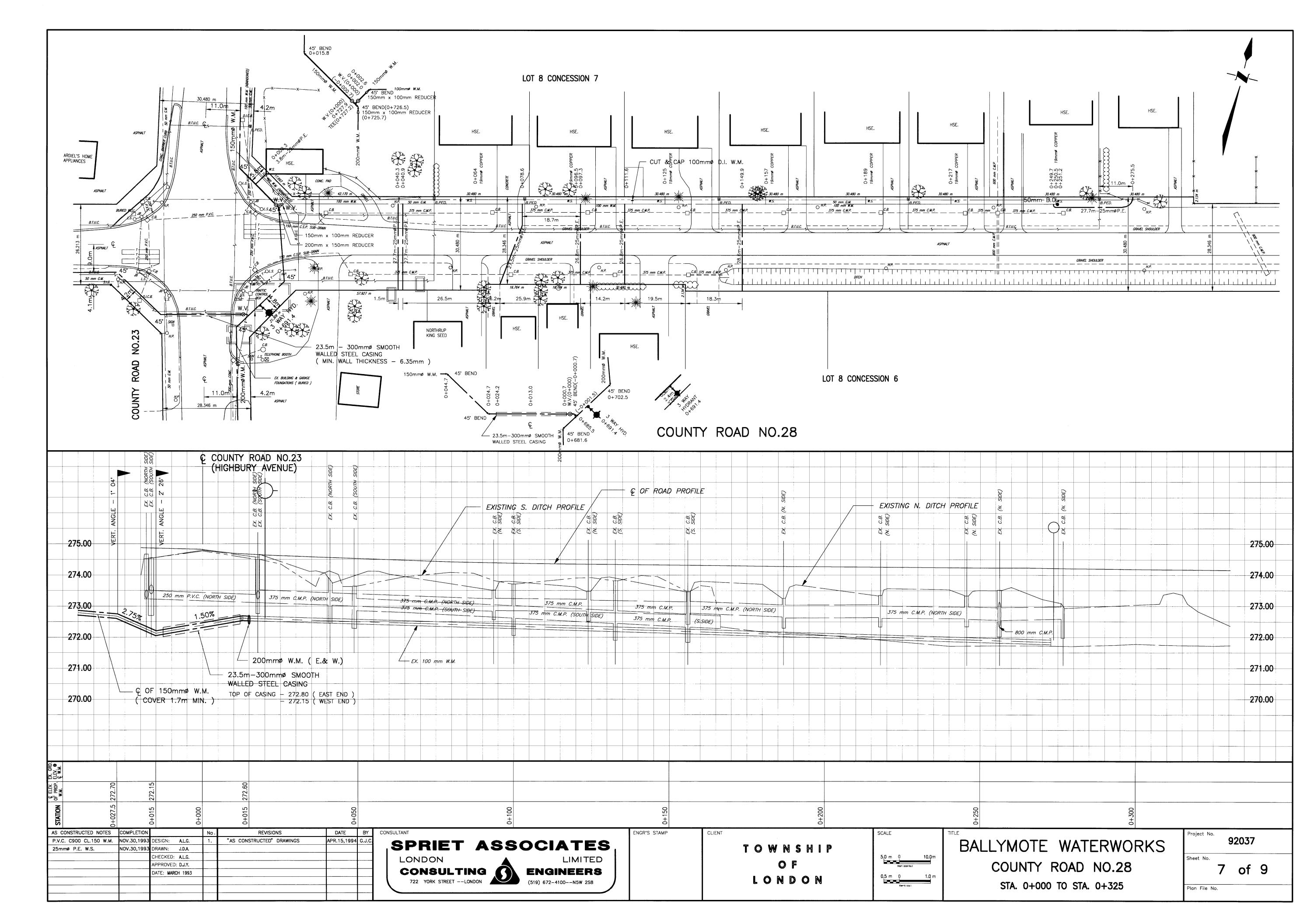
APPENDIX A

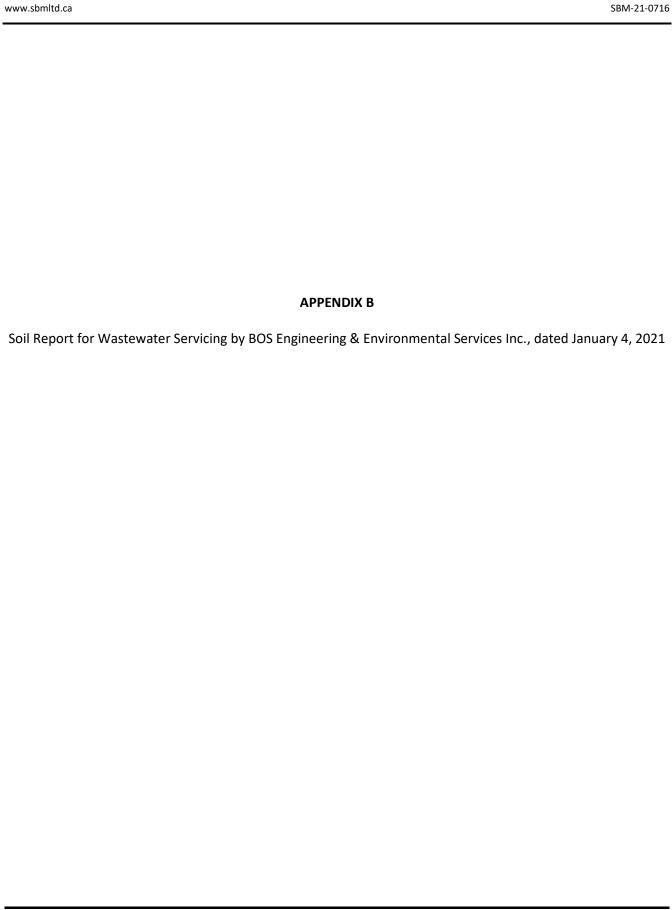
As-Constructed drawing from the Municipality, Project No. 92037 sheets 1, dated April 15, 1994 As-Constructed drawing from the Municipality, Project No. 92037 sheets 2, dated April 15, 1994 As-Constructed drawing from the Municipality, Project No. 92037 sheets 6, dated April 15, 1994 As-Constructed drawing from the Municipality, Project No. 92037 sheets 7, dated April 15, 1994











SOIL TESTING FOR WASTEWATER SERVICING

Proposed Lot Divisions

Concession 6 N Part Lot 8

(Geographic Township of London)

Municipality of Middlesex Centre County of Middlesex

Prepared for:

Brock Development Group Inc.
356 Oxford St E
London ON
N6A 1V7
519-281-6769, 519-697-0511
michelle@brockdg.com, shawn@brockdg.com

By:

BOS Engineering & Environmental Services Inc. 46 Donnybrook Rd. London Ontario, N5X 3C8 TEL: (519) 850-9987 EMAIL: a.bos@sympatico.ca

January 4, 2021

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1. INTRODUCTION

This report presents the results of a soils and sewage system assessment carried out at Municipal Address: 21488 Highbury Ave in the Municipality of Middlesex Centre, County of Middlesex. The property is described as Concession 6 N Part Lot 8 (Geographic Township of London) and is located in Ballymote. The 35 ha (86.4 ac) parcel is currently vacant. Frontage of all lots are to be onto Medway Road and Highbury Avenue North with 5 lots along Medway Road and 14 lots along Highbury Avenue. Proposed lots are will range in width from approximately 23m to 26m. Lot depths will be approximately 76 to 78m with an average lot size of 0.18 ha leaving a retained parcel of approximately 32.4 ha. Appendix A contains a map of the site location.

The lots are currently located on farm land. There are not any defined drainage patterns or roadside ditches along Highbury Avenue. A large road side ditch is present along Medway Rd.

The lots are currently proposed to be serviced with onsite wastewater treatment systems systems and private wells.

2. EXISTING SURFICIAL SOILS

Soil testing was conducted on November 26, 2020. A map of the site and test pit locations is presented in Appendix A together with soil test pits logs at nine (9) locations across the site.

The test pits were formed to depths of 1.4 to 1.5m. In all cases the underlying soils were comprised of clay to silty clay TILL with estimated soil percolation time in excess of 50 min/cm. Topsoil was generally 25 to 30 cm thick. At five (5) of the test pits there was a thin layer (22 to 46cm thick) of sand to sandy silt at varying shallow depths. Some of these layers (at TP 1 & TP 2) appeared to be pockets since they were water-bearing. The sandy layers were sampled and tested. Grain size analyses and classifications are presented in Appendix B. Assessment of the predominantly heavy soils was made in the field. Approximate soil test locations and test pit logs are presented in Appendix A.

Due to the variable depths and inconsistency of the sandy layers, a percolation time (T) of greater than 50 min/cm is recommended at this site for septic system design.

3. PROPOSED WASTEWATER TREATMENT SYSTEMS

In order to size the proposed wastewater treatment system, it is necessary to make assumptions regarding the house characteristics. In this case, a daily load of 3000 L/day was assumed in line with actual homes at a similar recent development in Bryanston. This assumed loading can include a 4-bedroom home with 300m2 of living area and up to 40 fixture units as outlined in Appendix E.

Based on the surficial soil as documented in section 2, conventional raised beds were sized requiring a minimum sand footprint of 750 m² or 23m wide x 33m deep. Therefore, a conventional raised bed would consume the entire rear yards of the lots. In Middlesex Centre, during lot creation, it is also required to designate an area for a second "contingency bed" for reconstruction in the event of bed failure. The proposed lot sizes are too small to allow this.

Therefore, enhanced pre-treatment (to CAN/BNQ 3680-600 standard) is required, allowing smaller "Type A" beds (approximately14m x 28m) as indicated on the sketch in Appendix E. This allows for a contingency bed that will partly overlap the primary bed area.

In order to attain the minimum setbacks from wells to septic systems, the water supply wells will be located in the front yards of the lots with the septic systems in the rear yards. The raised area location and direction of the proposed sand mantle drainage is dependent on both the existing topography and the proposed grading of the lots. There was not a topographical plan available at the time of reporting.

4. SEWAGE IMPACT ASSESSMENT

MOE Procedure D5-4 outlines a multi-step process to gauge the effects of the combined effluent discharges from all of the individual sewage systems in a development based on nitrogen as an indicator of groundwater impact potential.

4.1 Minimum Lot Size

Generally, if the average lot size is smaller than 1.0 ha in size with no lot being smaller than 0.8 ha, then a hydrogeological assessment is not required provided that the area is not hydrogeologically sensitive. This exemption does not apply to the subject lots as they are to be 0.18 ha in size.

4.2 System Isolation Considerations & Well Records

Where smaller lots than 1.0 ha are proposed, it is necessary to consider the status of isolation of the sewage effluent from the existing or potential supply aquifer. As with other lots in this area, the lots are to be serviced with private wells and on-site wastewater treatment systems to current OBC standards. Hence review of water well records for the area were reviewed to verify isolation and/or determine potential impacts of the sewage effluent on area wells.

There were 18 valid well records within approximately 500m of the proposed development site, 3 of which were professionally closed and sealed. Well locations and logs are presented in Appendices C and D, respectively. Following is a summary of the well logs:

Well #	Year Formed	Casing dia(cm)	Water Depth (m)	Pump Rate (lpm)	Soil Profile (m)	Status
4102089	1964	91	7.9	227	0 – 2.4 Sandy Clay 2.4 – 7.9 Hard Blue Clay	On highbury adjacent to proposed lots
4102091	1957	15	25.6	1365	0 – 1.3 Topsoil/Fill 1.3 – 2.4 Sand & Clay 2.4 – 22.6 Blue Clay 22.6 – 25.0 Hard Pan 25.0 - 26.2 Sand & Gravel	
4102092	1962	91	3.1	22.8	0 – 0.30 Topsoil 0.3 – 1.5 Sand 1.5 – 3.05 Blue Clay	185m west of highbury on medway
4102096	1967	91	3.7	13.7	0 – 2.4 Sand 2.4 – 3.7 Hard Blue Clay	110m west of highbury on medway
4102166	1959	12	42.4	26.5	0 - 3.1 Topsoil & Sandy Clay 3.1 - 9.1 Hard Pan 9.1 - 13.7 Sand 13.7 - 22.9 Clayey Sand & Sandy Hard Pan 22.9 - 27.4 Clay 27.4 - 30.2 Sand 30.2 - 42.4 Hard Pan & Clay 42.4 - 42.7 Sand/Gravel	
4105504	1971	91	5.5	9.1	0 – 1.5 Br. Clay 1.5 – 5.5 Blue Clay 5.5 – 5.8 Gravel 5.8 – 8.5 Blue Clay Till	at monitoring wells on petrocan property
4106814	1974	12	18.3	45.5	0 – 4.6 Br. Clay 4.6 – 13.7 Blue Clay 13.7 – 20.4 Gravel	
4107096	1974	12	29.6	9.1	0 – 5.5 Br. Sand 55 – 29.6 Blue Clay Till 29.6 – 30.2 Gr. Sand 30.2 – 31.7 Blue Clay	

4107563	1976 1994 Closed	15	63.3	n/a	0 – 2.7 Sand 2.7 – 47.5 Gr. Clay 47.5 – 81.4 Blue Clay 22.6 – 81.4.0 Hard Pan	Closed Sealed (sulphur)
4107571	1976	15	10.1	27.3	0 – 3.1 Red Clay 3.1 – 9.1 Layered Clay & Gravel 9.1 – 10.1 Porous Gravel	
4108667	1978	12	9.4	36.4	0 – 9.4 Br. Clay 9.4 – 9.4 Sand & Clay 9.4 – 11.3 Br. Sand	
4110852	1987	12	23.8	46	0 – 1.5 Br. Clay 1.5 – 23.5 layered Sand & Clay 23.5 – 23.8 Gr. Gravel	
4111987	1989	15	30.2	27.3	0 – 2.7 Br. Sand 2.7 – 16.2 Gr. Clay 16.2 – 24.4 Clay -layered sand & silt 24.4 – 30.2 Gr. Clay 30.2 – 31.7 Gr. Sand 31.7 – 32.9 Gr.Clay	
4112242	1990	12	21.3	54.6	0 – 0.91 Br. Clay 0.91 – 18.3 Gr. Clay 18.3 – 21.3 Bl. Sand	
4112352	1991	15	14.9	45.5	0 – 3.7 Br. Sand & Clay 3.7 – 14.6 Gr. Clay & Sand 14.6 – 14.9 Gr. Sand	
4114496	2000		14.9		unknown	Closed Sealed
7045068	2007	2	4.6	n/a	0 – 0.9 Br. Sand Till 0.9 – 4.6 Gr. Clay & Sand 14.6 – 14.9 Gr. Clay till	Monitoring Well (5 wells)
7304894	2017	107	3.1		unknown	Closed Sealed

The soil profiles on the well logs are generally consistent with the shallow test pit data. Most non-closed deep wells are to an aquifer ranging from 9.4m to 30.2m in depth. There appears to be a clay overburden with some sand or silt lenses over the deeper aquifer with considerable unlayered clay to prevent migration of wastewater effluent. With the confined aquifer, wastewater effluent is expected to migrate to surface drainage swales and ditches where denitrification will occur.

There are four (4) shallow wells in proximity of the site, namely wells 4102092, 4102096, 4102089, and 4105504. The first two wells are approximately 185 m and 110m respectively west of Highbury Avenue and hence are unlikely to be affected by the development. However, well 4105504 on the Gas station property southeast of the intersection of Highbury and Medway Roads is shallow and adjacent the development. The status of this well should be examined since there is an existing deeper drilled well on the same property. Well 4102089 is also shallow and immediately adjacent the south end of the proposed

development and its status should therefore also be examined. Both of these shallow wells should be decommissioned and replaced, if not done already.

One deep well to the bedrock aquifer was decommissioned due to sulphur content.

Current pump rates for existing deeper wells range from 9 to 46 Litres/minute.

5. **SUMMARY & RECOMMENDATIONS**

To facilitate this development, it is recommended that:

- 1. Two shallow wells (Well 4105504 and Well 4102089) adjacent the development should be decommissioned (if not done already) and serviced with a replacement water supply, if necessary.
- 2. A preferred water supply for all of the new lots would be municipal water, if feasible.
- 3. The proposed development can accommodate sewage design loads of 3000 L/day on each lot as outlined in Appendix E.
- 4. Deep drilled wells should be located in the front yards of the development with septic systems and contingency areas in the rear yards.
- 5. Enhanced treatment of sewage (to CAN/BNQ 3680-600 standard) is required to facilitate use of smaller "Type A" distribution beds to fit the proposed lot sizes. These pre-treatment units will also reduce risks of shallow aquifer contamination.
- 6. The Ontario Building Code does apply to the sewage system construction.

The proposed sewage systems will be required to meet all regulations and required setbacks from wells outlined in Part 8 of the Ontario Building Code and CAN/BNQ 3680-600 standard.

7. Building permits will require lot grading plans and specific septic system designs for the individual lot developments.

BOS Engineering & Environmental Services Inc.

DRAFT

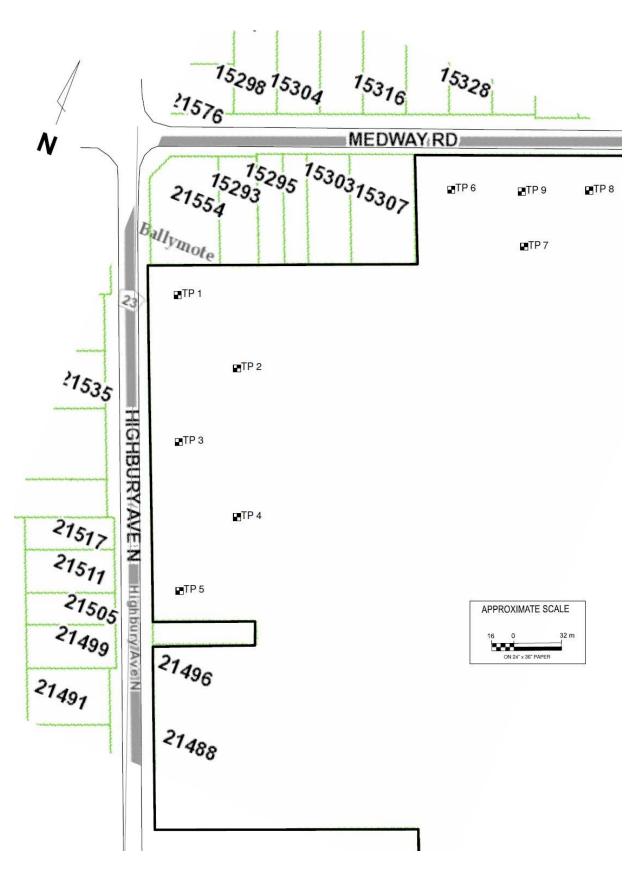
Art W. Bos, P.Eng.

Encl

- Appendix "A" Map: Soil Test Locations & Logs
- Appendix "B" Soil Grain Size Analysis of Selected Samples
- Appendix "C" Map: Existing Water Well Records
- Appendix "D" Individual Well Records (Provincial Database)
- Appendix "E" Wastewater Treatment System Assumptions & Sizing

Appendix A

Map: Soil Test Locations & Logs



SITE SOIL INFORMATION (BOS ENGINEERING – Nov 26, 2020)

<u>TEST</u>	DEPTH (cm)	SOIL TYPE
TP 1	0 - 30 30 - 76 76 - 114 114 - 152	TOPSOIL Hard Silty CLAY Sand (Tested: T = 8 min/cm) Gr. CLAY (T > 50 min/cm)
TP 2	0 - 30 30 - 76 76 - 152	Seepage @ 114 cm TOPSOIL Mottled Sandy SILT (Tested: T = 40 min/cm) Clay TILL (T >50 min/cm) Seepage @ 76cm
TP 3	0 - 30 30 - 81 81 - 137	TOPSOIL Mottled Silty CLAY (sand pockets) CLAY TILL (T >50 min/cm) No Seepage
TP 4	0 - 27 27 - 69 69 - 132	TOPSOIL Mottled Silty CLAY Gravelly CLAY TILL (cobbles) (T >50 min/cm) No Seepage
TP 5	0 - 30 30 - 56 56 - 127	TOPSOIL Mottled Silty CLAY Stoney Clay TILL (T >50 min/cm) (100mm TILE @ 81 cm) No Seepage
TP 6	0 - 30 30 - 41 41 - 81 81 - 107 107 - 140	TOPSOIL Silty CLAY Sand (Tested: T = 10 min/cm) Gr. SAND Clay TILL (T >50 min/cm) Seepage @ 81 cm
TP 7	0 - 30 30 - 46 46 - 122	TOPSOIL Gr. CLAY Gr. Mottled Silty CLAY (T >50 min/cm) No Seepage
TP 8	0 - 25 25 - 48 48 - 76 76 - 127	TOPSOIL Gr. CLAY SAND CLAY TILL (T >50 min/cm) No Seepage
TP 9	0 - 25 25 - 69 69 - 91 91 - 132	TOPSOIL Gr. Mottled CLAY Gravelly SAND (T = 10 min/cm) CLAY TILL (T >50 min/cm) No Seepage

Page 11 Wastewater Assessment

Appendix B

Soil Grain Size Analysis of Selected Samples

BOS Engineering Environmental Services

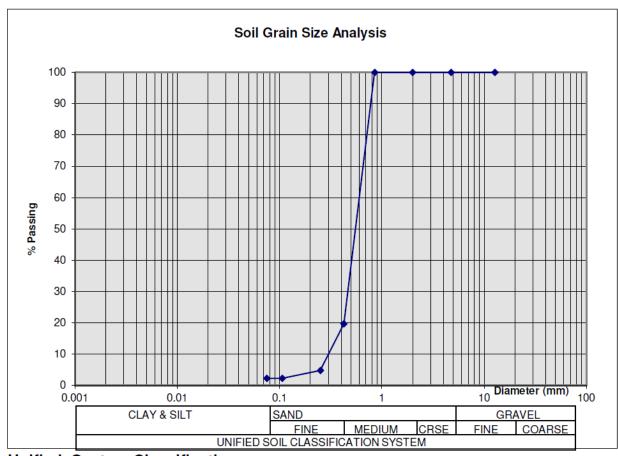
Project :Ballymote LotsClient :Brock DevelopmentTest Pit :TP1RE:Waste Treatment System

 Depth :
 76 to 114 cm
 Proj. No.
 2011-22

 Dry Mass:
 117.0 g
 Date:
 Nov 27 20

CHART DATA

Sieve No.	Mass (Cum. Mass	Diam. (d)	% Passing	
		0	12.7	100	
4	0.0	0	4.75	100	
10	0.0	0	2	100	
20	0.0	0	0.85	100	
40	94.0	94	0.425	20	
60	17.4	111.4	0.25	5	
140	2.9	114.3	0.106	2	
200	0.0	114.3	0.075	2	



Unified System Classification:

SP Poorly Graded SAND (2% Finer than No. 200 sieve)

Est. Percolation Time: T = 8 min/cm

BOS Engineering Environmental Services

 Project :
 Ballymote Lots
 Client :
 Brock Development

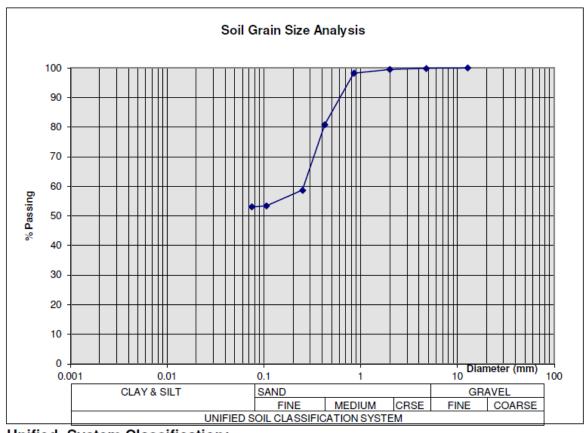
 Test Pit :
 TP2
 RE:
 Waste Treatment System

 Depth :
 30 to 81 cm
 Proj. No.
 2011-22

 Dry Mass:
 124.9 g
 Date:
 Nov 27 20

CHART DATA

Sieve No.	Mass	Cum. Mass	Diam. (d)	% Passing	
		0	12.7	100	
4	0.2	0.2	4.75	100	
10	0.4	0.6	2	100	
20	1.6	2.2	0.85	98	
40	21.7	23.9	0.425	81	
60	27.7	51.6	0.25	59	
140	6.6	58.2	0.106	53	
200	0.4	58.6	0.075	53	



Unified System Classification:

Sandy SILT (53% Finer than No. 200 sieve)

Est. Percolation Time: T = 40 min/cm

BOS Engineering Environmental Services

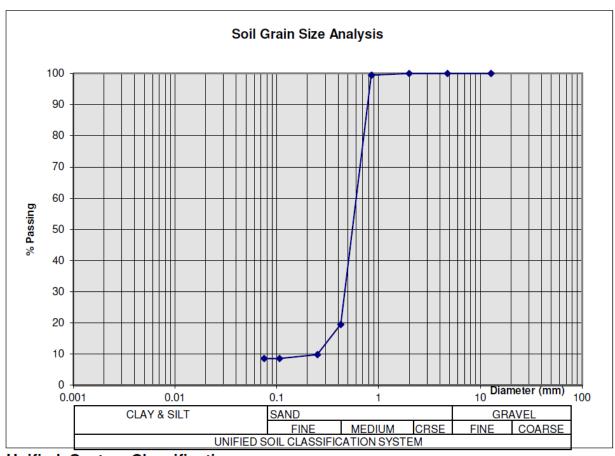
Project :Ballymote LotsClient :Brock DevelopmentTest Pit :TP6RE:Waste Treatment System

 Depth :
 41 to 81 cm
 Proj. No .
 2011-22

 Dry Mass:
 134.2 g
 Date:
 Nov 27 20

CHART DATA

Sieve No.	Mass (Cum. Mass	Diam. (d)	% Passing	
		0	12.7	100	
4	0.0	0	4.75	100	
10	0.0	0	2	100	
20	0.7	0.7	0.85	99	
40	107.3	108	0.425	20	
60	13.0	121	0.25	10	
140	1.7	122.7	0.106	9	
200	0.0	122.7	0.075	9	



Unified System Classification:

SM Sand Trace Silt (9% Finer than No. 200 sieve)

Est. Percolation Time: T = 10 min/cm

Appendix C

Map: Existing Water Well Records



Appendix D

Individual Well Records

County of Therete		EC(ORD	JUN 11 de RESOLOGY LON DONNEY month Deva	. X00.
Casing and Screen Record			Pympin	g Test	
Inside diameter of casing Total length of casing Type of screen Length of screen Depth to top of screen Diameter of finished hole	Pumping I Duration of Water clea	oing rates of test par or clanded partes	oumping /	f test Class	G.P.M.
Well Log					Record
Overburden and Bedrock Record	Fro		To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
- de aldad	0		8"	2611	press
hard blue clay	6		26		
			L .	6 M. II	
For what purpose(s) is the water to be used? Is well on upland, in valley, or on hillside? Drilling or Boring Firm Address. Licence Number 52 Name of Driller or Borer Address Date. (Signature of Licensed Drilling or Boring Contractor) Form 7 15M-60-4138	In ros	ad and	ım below sho	or of Well w distances of we ndicate north by the state of the state	JS'OU
OWRC COPY				CSS.S8	
					

	∑ N The Wat De	Well	JAN 30 19 GEOLOGICAL B ALEPASSMENT O Mines RECOT O, Village, Town or C Idress	d City	2091
(day)	(month)	(year)			
Pipe and Casing	Record			Pumping Test	
Casing diameter(s) Length(s) Type of screen Length of screen	Pi	atic level	1,300	G. P.F.	
Well Log				Water Record	
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	No. of feet water rises	Kind of water (fresh, salty, or sulphur)
top sail		- 3			
if he and & clay	6	8 24			
flye clay - broudles	8	82			-
party gavel	82	86	52	842	frist
For what purpose (s) is the water to the water the same of the same of the water the same of the water the	nillside?		In diagram below	ocation of Well r show distances of e. Indicate north	by arrow.
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Name of Driller Address Licence Number. I certify that the forstatements of fact a	oregoing		·~ (13311 12mill	

Con. 3 6 Lot.	Townsl	REC	Act ORD www.or City	OTTOR ON A SECOND WATER OTTOR ON A SECOND OF A SECOND	2092 N TWF 1962
Casing and Screen Record			Pumping	Test	
Inside diameter of casing Total length of casing Type of screen Length of screen Depth to top of screen Diameter of finished hole	Te Pu Du	st-pumping ramping level ration of test later clear or cl	pumping Loudy at end of	les test Claur	G.P.M G.P.M
Diameter of finished note	wi	th pump setti	ng of £	feet belo	w ground surfac
Well Log				Water	r Record
Overburden and Bedrock Record		From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
Lon soil		0	/ ව	4	fileti
For what purpose(s) is the water to be used? Is well on upland, in valley, or on hillside? Italiado Drilling or Boring Firm Tay Hudso Address Licence Number Name of Driller or Borer Toy Hudso Address Date Joy Hudso (Signature of Licensed Drilling or Boring Contractor) Form 7 15M Sets 60-5930 OWRC COPY		In diagraroad and	Location am below show lot line. Inc	distances of we licate north by	Huffmy and
Wastewater Assessment				Pa	ge 21

UTW 17 2 480 380 E				,
Elev HR 0895 WATER WEI Basin 239 MI DLESEN County or District Lot 9	LL RECO	ORD own or City	May	1967 1967
Casing and Screen Record	Address	Pumping		
Inside diameter of casing 36 37 Total length of casing 12 ft Type of screen Length of screen Depth to top of screen Diameter of finished hole 36 5 Well Log Overburden and Bedrock Record Sondy clay Land bluke clay	1	te 3 12 ft umping 1 oudy at end of umping rate	As test Clear 3	G.P.M. G.P.M. ow ground surface r Record Kind of water
For what purpose(s) is the water to be used? Is well on upland, in valley, or on hillside? Drilling or Boring Firm Pay Thudson Address Licence Number / 7 Name of Driller or Borer front Address Date 1961 (Signature of Licensed Drilling or Boring Contractor) Form 7 15M-60-4138 OWRC COPY	In diagrar		distances of we dicate north by	

Basin 3 40			ission Act, 1957 RECORI	2007 URG28 4	1360 2166 LIMISSIUM
County or District MIDDLE S.E.X					
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Casing and Screen Record			Pum	ping Test	
Depth to top of screen		Test-pun Pumping Duration Water cl	nping rate	20/f 4 hrs	C.P.M.
Diameter of finished hole 5 2			ended pumping r pumping level of	1000	G.P.M.
Well Log		1		ter Record	
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	No. of feet water rises	Kind of water (fresh, salty, sulphur)
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For what purpose(s) is the water to be used? Is well on upland, in valley, or on hillside?	housl	,	n diagram below oad and lot line		
Drilling Firm Harveld Bregs Address B. B. 5. Licence Number 3.67 Name of Driller Harveld Stage Address B. B. 5. Jondon Date May 5. 19 (Signature of Licensed Drilling Contractors)	is!	LOT 9	450 con 1450 th	ondon L	aconship
Form 5 15M-58-4149			14	CSS.S8	

			The Ontario	Water Res	ources	Comm	ission	Act		HOP,	38
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	Water management in	Ontario 1. PRINT ONLY IN SE	ACES PROVIDED	11	141	05504	+ -	MUNICIP. 4,10,0	o con.	24	1 10
	COUNTY OR DISTRICT	2. CHECK X CORRE	TOWNSHIP, BOROUGH,	1 2	L		CON.	BLOCK, TRACT, SI	IRVEY, ETC.	//VI	22 23 2 OT 25-27
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Ţ		LO	G OF OVERBURDE	N AND BEDI	ROCK A	MATERIAL	.\$ (SEE	INSTRUCTIONS)			
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	20-23	SALTY 4 MINERAL FRESH 3 SULPHUR	17-18 □ STEEL 2 □ GALVANIZEI	19		20-23	DEPTH :	SET AT - FEET	MATERIAL AND	TYPE (CE	MENT GROUT, PACKER, ETC.)
	25.2A	SALTY 4 MINERAL FRESH 3 SULPHUR	3 ☐ CONCRETE 4 ☐ OPEN HOLE			27-30		-13 14-17			
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8	Ministry of the Environment
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	SPACES PROVIDED 11	4107563 41008 60N 167
COUNTY OR DISTRICT	TOWNSHIP, BOROUGH, CITY, TOWN VILLAGE	
	0 # , ^	DATE COMPLETED 41-53
	ING	ICVA ONTARIO DAY 27 MO OLO VR 94
10 12 L	OG OF OVERBURDEN AND BEDF	ROCK MATERIALS (SEE INSTRUCTIONS)
GENERAL COLOUR COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION DEPTH - FEET FROM TO
	ABANDONED	
	()all # 111-	7563
	Well 41-	1565
31 32		
41 WATER RECORD	51 CASING & OPEN HOLE	RECORD Z 15120 NO 10241106 31-32 01AMCTR 34-33 LINGTH 35-03
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2 SALTY 6 GAS	24-25 1 DSTEEL 26	367 170 grave / leadply 170
2 GALTY 6 GAS		7" O" concrete mix (dry)
71 PUMPING TEST METHOD 10 PUMPING RATE	D-14 DURATION OF PUMPING 15-16 17-18 GPM HOURS MISS	LOCATION OF WELL
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<u>₩</u>	29-31 32-34 35-37	
IF FLOWING. 38-81 PUMP INTAKE S		
SHALLOW DEEP SETTING	43-45 RECOMMENDED 46-49 PUMPING FEET RATE GPM	
34		
FINAL STATUS 1	S ABANDONED, INSUFFICIENT SUPPLY B ABANDONED POOR QUALITY 7 UNFINISHED	
OF WELL 4 RECHARGE WELL	DEWATERING 5 COMMERCIAL	
WATER STOCK STOCK	6 MUNICIPAL 7 PUBLIC SUPPLY 0 COOLING OR AIR CONDITIONING	
O OTHER	9 □ NOT USED	
METHOD DX CABLE TOOL	6 BORING ONAL) 7 DIAMOND	
CONSTRUCTION - ROTARY (AIR) 5	9 DRIVING DIGGING OTHER	DRILLERS REMARKS 132221
NAME OF WELL CONTRACTOR	WELL CONTRACTOR'S	
Stainton's Ltc	d. 4876	SOURCE ST CONTINUED STATE OF INSPECTION AND STATE OF I
NAME OF WELL TECHNIZIAN	Ontario WELL TECHNICIAN'S LICENCE NUMBER	D STEMBES ORIGINAL WATER WELL RECORD
SIGNATURE OF JECHNICIAN/CONTRACTOR	SUBMISSION DATE DAY 30 MO 06 VR 94	(410 7563) ATTACHED, AUG. 23/94-48.
MINISTRY OF THE ENVIRONM	DAY 30 NO OE VR.94	CSS.S8 FORM NO. 0506 (11/86) FORM 9

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MINISTRY OF THE ENVIRONMENT The Ontario Water Resources Act WATER WELL RECORD 40 P/3B

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		2 D	/ //	3	_		<i>-</i>	DATE COMPLETED	"·"
		7.68	100		LEVATION	*	BASIN CODE	DAY 26 NO 0	3vr76
		OC OF OVERBURDEN	100	75	5.8.9.6	10	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
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brown	sand				fi	18		0	8
black	send				-			8	
grey	cloy	sand and st	ones		2114	eduim		9	1 56
black	limestone							156	164
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	TER RECORD	51 CASING & O		RECO DEPTH		I III I	S) OF OPENING 31-	33 DIAMETER 34-38	LENGTH 39-40
WATER FOUND AT - FEET	KIND OF WATER	DIAM MATERIAL INCHES	WALL THICKNESS INCHES	FROM	10	I W	RIAL AND TYPE	DEPTH TO TOP OF SCREEN	FEET 41-44 80
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25-28	FRESH 3 SULPHUR 29	4 OPEN HOLE		57 0	207		0-13 14-17 1-21 22-25		
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/R711 /	2 MAILER 0005		, OO 17-18			L	OCATION OF	WELL	
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Z IF FLOWING.	FEET FEET 38-41 PUMP INTAKE S						,		
RECOMMENDED PUM	GPM RECOMMENDED	FEET 1 2 CLEAR 43-45 RECOMMENDED	2 CLOUDY]					
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50-53	GPM./FT. SPEC	IFIC CAPACITY		1	ب	079	ا لا المما	of ? con!	VII
FINAL STATUS	WATER SUPPLY OBSERVATION WELL		CIENT SUPPLY		7>1	,	+ J50'		
OF WELL	3 ☐ TEST HOLE 4 ☐ RECHARGE WELL	7 UNFINISHED			1.41		Promite.		- 1
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	OTHER	* □ NOT U.	SED				18		
METHOD OF	CABLE TOOL ROTARY (CONVENTION ROTARY (REVERSE)	● □ BORING ONAL) 7 □ DIAMOND				•	#23		
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	un fors		<u>3</u> _{v∗} 76	0				CSS.S8 W	rric
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	767,680 H	ELEVATION 2	BASIN CODE	IV
1 1 1	LOG OF OVERBURDEN AND BEDRO	CK MATERIAL	S (SEE INSTRUCTIONS)	
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Grey Gravel			Forous	50 33
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AT - FEET	INCHES INCHES	TO 13-16	S MATERIAL AND TYPE	DEPTH TO TOP 41-44 80 OF SCREEN FEET
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20-23 1 FRESH 3 SULPH	JR 24 17-18 1 ☐ STEEL 19 2 ☐ GALVANIZED	20-23	FROM TO	AND TYPE (CEMENT GROUT, LEAD PACKER, ETC.)
2 SALTY 4 MINER	UR 29 4 OPEN HOLE	27-30	10-13 14-17 18-21 22-25	
2 SALTY 4 MINER	2 ☐ GALVANIZED UR 34 80 3 ☐ CONCRETE		26-29 30-93 RO	
2 SALTY 4 MINER	ING RATE II-14 DURATION OF PUMPING	N	LOCATION OF W	ELL
TI PUMP 2 BAILER (0006 GPM 48 15-16 0 07-18 HOURS MINS	A IN DI	AGRAM BELOW SHOW DISTANCES OF W	
LEVEL END OF PUMPING	WATER LEVELS DURING 2 ☐ RECOVERY MINUTES 30 MINUTES 45 MINUTES 60 MINUTES	LOTI	INE. INDICATE NORTH BY ARROW.	
		con	जा	•
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SHALLOW DEEP SETT	INGU 25 FEET PUMP 10006 GPM	con	VI.	#
	/FT. SPECIFIC CAPACITY S □ ABANDONED, INSUFFICIENT SUPPLY]	(°-21	bmile.
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OF (3 G ROTARY 6 DRILLING	AIR) 9 DRIVING	DRILLERS REMAI	RKS:	
NAME OF WELL CONTRACTOR	LICENCE NUMBER	T DATA	58 CONTRACTOR 59-62 DATE RE	CCEIVED 63.60 10
E John Wish	m Well Drilling 15466	DATE OF INS	STOR INSPECTOR	700578 2
ANTE OF DRILLER PH BORER PH	eld, Ont. NOLOJO	S AY /	PITLESS MYNOTER.	P/mg
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State Stat			CASING & OPEN HOLE RE		Tom 07
Secondary Seco	AT - FEET		DIAM MATERIAL THICKNESS FROM	TO MATERIAL AND TYPE	
CONTINUED CONT			GALVANIZED JUY	74	ALING RECORD
SALTY WINDERS	2 0	_ SALTY 4 _ MINERAL	4 G DPEN HOLE	DEPTH SET AT - FEET MATERIAL	
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STATUS OF WELL STATUS OF WELL	STATIC		GPM OB HOURS MINS	IN DIAGRAM BELOW SHOW DISTANCES OF WE LOT LINE. INDICATE NORTH BY ARROW.	LL FROM ROAD AND
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FINAL STATUS OF WELL STATUS O	SIVE RATE			6	
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COUNTY OR DISTRICT	TOWNSHIP, BOROUGH, CITY TOWN, VILLAGE	CON BLOCK TRACT SURVEY ETC.	9
		Ontario NOM 1CO DAY 24	етер 44.53 мо_6 ун. 87
	767220 1°	2880 SC BASIN CODE	
<u> </u>	OF OVERBURDEN AND BEDROC	MATERIALS (SEE INSTRUCTIONS)	.,
GENERAL COLOUR COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH FEET
COMMON MAILANCE			0 1
Black Topsoil Brown Clay	Sand		1 5
Grey Clay	Sand and Gravel	Layered	5 77
Grey Gravel		Pine	77 78
31			
32	51 CASING & OPEN HOLE R	SIZE(S) OF OPENING 31-33 DIAMET	ER 34-34 LENGTH 39-40
WATER RECORD WATER FOUND AT - FEET KIND OF WATER		EPTH - FEET	INCHES FEET DEPTH TO TOP 41-44 10 OF SCREEN
78 2 SALTY 6 GAS	10-11 STEEL 12 2 GALVANIZED	13-16	OF SCREEN FEET
15-18 1 FRESH 3 SULPHUR 15 4 MINERALS SALTY 6 GAS	5 3 CONCRETE 4 COPEN HOLE 5 PLASTIC 188 0	78 61 PLUGGING & SEAL	
20-23 FRESH 3 SULPHUR 24 4 MINERALS	17-18 DSTEEL 19 2 GALVANIZED 3 CONCRETE 4 OPER HOLE	20-23 DEPTH SET AT - FEET MATERIAL AND FROM 10 10-12 54-17	TYPE (CEMENT GROUT, LEAD PACKER, ETC.)
25-28 1 FRESH 3 SULPHUR 29	4 GOPEN HOLE 5 PLASTIC 24-25 1 GSTEEL	27-30 18-21 22-25	
30-33 FRESH 3 SULPHUR 34 40	2 GALVANIZED 3 GONCRETE 4 GOPEN HOLE 5 DPLASTIC	26-29 30-33 80	
PUMPING TEST METHOD 10 PUMPING RATE	5 Liphastic	LOCATION OF WEL	L
71 PUMP 2 BAILER	10 GPH 1 15-16 0 17-16 HOURS 0 MINS	IN DIAGRAM BELOW SHOW DISTANCES OF WELL	
LEVEL PUMPING WATER LE	VELS DURING # RECOVERY	LOT LINE INDICATE NORTH BY ARROW	
19 24 19 36 18 18 NINUTES 19 24-18 36 18 18 NINUTES 19 24-18	36 net 36 net 36 net		
24 19-21 36 22-24 35 MINUTES 24 7EET 36 7EET 3		Vet 7+h	*
RECOMMENDED PUMP TYPE RECOMMENDED PUMP SHALLOW DEEP SETTING		1 SOKM	
30-53		×	
FINAL STATUS 54 1		207	
OF WELL A RECHARGE WELL	7 □ UNFINISHED 9 □ DEWATERING	1 644	
11-36 ⊕ DOMESTIC 2 ☐ STOCK WATER 3 ☐ IRRIGATION	5 COMMERCIAL 6 MUNICIPAL 7 PUBLIC SUPPLY	ر مر	
USE INDUSTRIAL OTHER	DOCUMENT OF AIR CONDITIONING TO NOT USED	4	
57 L CABLE TOOL	4 ☐ BORING	7	
METHOD OF CONSTRUCTION ROTARY (CONVENT ROTARY (REVERSE ROTARY (AIR)	TIONAL) 7 DIAMOND) B DETTING 9 DRIVING		14401
s 🗋 AIR PERCUSSION	☐ DIGGING ☐ OTHER	DRILLERS REMARKS DATA 58 CONTRACTOR 59-62 DATE RECEIVE	
Mervin Jones	WELL CONTRACTOR'S LICENCE NUMBER 3009	SOURCE	L 1 5 1987
Mervin Jones ADDES R. R. #3 Thorndale NAME OF WELL FECHNICIAN MITTAY S. JONES SHAMPHE OF TECHNICIAN/CONTRACTOR	o, Ontario NOM 2PO	DATE OF INSPECTION INSPECTOR	De)
NAME OF WELL TECHNICIAN	well technician's ligence number T=0068	DIT LESS	7
Murray S. Jones	SUBMISSION DATE	8 2,88 THESS CSS	.S8
MINISTRY OF THE ENVIRON	_ \		ORM NO. 0506 (11/86) FORM

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Ontario	SPACES PROVIDED CCT BOX WHERE APPLICABLE	4111987 <u>¼ 1009</u> (<u>co</u> n	22 21 24
COUNTY OR DISTRICT	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE	CON . BLOCK, TRACE, SURVEY ETC	LOT 23-27
PHIODLESEX	LONDON	6 DATE COMPLE	TED 48-53
	RR#I ARV		_ MO _ Y VR 89
21 7 480	3.7.6 4.7.6.7.9.8.9 "	CELVATION PC MASIN CODE	
1 2 10 12	OG OF OVERBURDEN AND BEDROO		
HOST	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET
GENERAL COLOUR COMMON MATERIAL			0 9
BROWN SANO	STONES		9 55
GREY CLAY			55 80
GREY CLAY	SILT, SAND, STE	NES .	80 99
GREY CLAY	3 TONES		99 104
GREY SANO	SIAT		104 108
GREY CLAY	STONES		101 100
0			
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31			
32 10 14 15		SIZE ST OF OPENING 31-33 DIAMETE	75 80 ER 34-38 LENGTH 39-40
WATER FOUND WATER FOUND AT - FEET KIND OF WATER	51 CASING & OPEN HOLE F	DEPTH FEET W	INCHES FEET
10-13 1 FRESH 3 SULPHUR 14 99 2 SALTY 4 MINERALS	INSIDE DIAM MATERIAL THICKNESS FRE	MATERIAL AND TYPE 13-16 70 75 75 75	DEPTH TO TOP 41-44 10 OF SCREEN 99 TEET
1	Li 30 CONCRETE ./88 + 2	99	
15-18 1 FRESH 3 SULPHUR 19 SALTY 4 MINERALS 6 GAS	5 PLASTIC		TYPE LEAD PACKER, ETC.)
20-23 1 FRESH 3 SULPHUR 24 4 MINERALS 2 SALTY 6 GAS	1/2 I STEEL 2 GALVANIZED 3 GONCRETE 4 GOPEN HOLE	10-13 14-17	LEAD PACKER, ETC.
25-28 1 FRESH 3 SULPHUR 29	5 PLASTIC /0	4 108	
2 SALTY 6 SINERALS 6	2 □ GALYANIZED 3 □ CONCRETE 4 □ OPEN HOLE	20-29 30-33 80	
	5 PLASTIC		\$1 4 .
71 PUMPING TEST METHOD 10 PUMPING RAT	6 GPM 9 HOURS 17-18	LOCATION OF WELL	
STATIC WATER LEVEL 25 END OF UMPING WATER	LEVELS DURING	IN DIAGRAM BELOW SHOW DISTANCES OF WELL F LOT LINE INDICATE NORTH BY ARROW.	ROM ROAD NO
19-21 22-24 15 HINUTES	$_{220}^{30}$ 30 MINUTES 45 MINUTES 60 MINUTES $_{220}^{32-31}$ 98 $_{5681}^{29-31}$ 102 $_{5681}^{32-34}$ 103 $_{5681}^{33-37}$		411
THE RECOMMENDED PUMP TYPE	E SET AT WATER AT END OF TEST 42	6 TH. Con.	1 11
GPM GPM	106 FEET 1 X CLEAR I CLOUDY		,
RECOMMENDED PUMP TYPE SHALLOW DEEP RECOMMENDED PUMP SETTING	ED 43-45 RECOMMENDED 46-45 PUMPING 6 GPM	554 260	,
50-53		140	113
FINAL 2 OBSERVATION W	■ ABANDONED, INSUFFICIENT SUPPLY ELL ■ BANDONED POOR QUALITY	12001	10
STATUS , TEST HOLE OF WELL 4 RECHARGE WELL	7 UNFINISHED		1 3
55-56 1 M DOMESTIC	5 COMMERCIAL		"
WATER 2 STOCK 3 IRRIGATION	MUNICIPAL PUBLIC SUPPLY COOLING OR AIR CONDITIONING	' '	15
USE INDUSTRIAL OTHER	P O NOT USED		
METHOD 2 ROTARY (CONVE	● □ BORING NTIONAL	·	M
OF 3 ROTARY (CONVE			28595
S AIR PERCUSSION	☐ DIGGING ☐ OTHER	DRILLERS REMARKS	
NAME OF WELL CONTRACTOR	WELL CONTRACTOR'S	SOURCE SI CONTRACTOR SP-62 DATE RECEIVED FEB	2 3 1990
STONER WELL GREAT	72	OATE OF INSPECTION INSPECTOR	,000
AGORESS RAPE TO STORY OF THE ST	OENTZELO. WELL TECHNICIAN'S	M AEMAPAS	
W. STONE	WELL TECHNICIAN'S	8	
SIGNATURE OF TECHNICIAN CONTRACTOR	SUBMISSION DATE DAY 20 NO. 4 YR. 89	CSS.	∴36
MINISTRY OF THE ENVIR		FO	RM NO. 0506 (11/86) FORM S

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MIDDLESEX	2. CHECK 🗵 COR	TOWNSH	IDON	TOWN, VILLA	GE		CON	BLOCK, TRACT, SURVEY	ETC		OT 25-27
OWNER (SURNAME FIR			R. R. #1	Arra	Onta	rio NOM	- 100)	DATE COMPL		ve 91
	DINGS INC.	515	MORTHING		RC	ELEVATION	IIC I	BASIN CODE		hi	ıv
21	17 480		47686		<u></u>	. 9,00	S	METALICATIONS)			
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GENERAL COLOUR	COMMON MATERIAL		OTHER MATE							0	1
Black	Topsoil	Clay								1	12
Brown Grey	Sand	Sand								12	48
Grey	Sand	-				Coar	se			48	49
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41 WA	TER RECORD	51	CASING &		LE RE	CORD	Z ISLO	S OF OPENING	31-33 DIAME		LENGTH 39.40
WATER FOUND AT - FEET	KIND OF WATER	INSIDE DIAM INCHES	MATERIAL	WALL THICKNESS INCHES	FROM	PTH - FEET H TO	SCRE	ERIAL AND TYPE		DEPTH TO TOP OF SCREEN	41-44 10
	FRESH 3 SULPHUR 4 SHIPHUR 4 SALTY 6 SALTY 6 SALTY	10-11	1 STEEL 2 GALVANIZED 3 GONCRETE 4 GOPEN HOLE	200	_	13-16					FEET
15-18 I	FRESH 3 SULPHUR 19 SALTY 6 GAS	6	3 LIPEASTIC	188	0	49	61 DEPTH	CET AT . EFFT		LING RECO	ORD ENT GROUT
20-23	FRESH 3 SULPHUR 24 SALTY 6 GAS	17-18	1 STEEL 2 GALVANIZED 3 CONCRETE 4 OPEN HOLE			20.23	FROM		MATERIAL AND	D TYPE LEAD F	ACKER, ETC
25-28 1	TRESH 3 SULPHUR 29 SALTY 6 GAS	24-25	5 LI PLASTIC	1		27-30	-	18-21 22-25			· · · · · · · · · · · · · · · · · · ·
30-33 1	FRESH 3 SULPHUR 34	10	2 GALVANIZED 3 GONCRETE 4 GOPEN HOLE 5 GPLASTIC				2	6-29 30-33 80			
PUMPING TEST M		J	1-14 DURATION OF P	UMPING				LOCATION) F WEL	L 18h	
71	Z BAILER	10	GPH 48 НО	JRS	17-14 . MINS	1N DI		LOW SHOW DISTANCE			AND
STATIC LEVEL	PUMPING	R LEVELS DUR	ING 2 🖂	PUMPING RECOVERY	ITES	LOT	INE IN	DICATE NORTH BY A	RROW.		
TES 7	23 23	6-28	29-31 32		35-37 FEET					•	TN
IF FLOWING GIVE RATE	38-41 PUMP INTAI	KE SET AT	WATER AT END		42						, .
RECONNENDED P	UMP TYPE RECOMMEN	33	FEET 1 ECLEAR 13-45 RECOMMENDED PUMPING		46-45						
SHALLO 50-53	W DEEP SETTING	35	FEET RATE	10	GPM	-		-			
FINAL	1 St WATER SUPPLY	s (ABANDONED. INSU	FFICIENT SUI	PPLY	744	Con.				
STATUS OF WELL	2 OBSERVATION V 3 TEST HOLE 4 RECHARGE WEL	, [ABANDONED POOR UNFINISHED DEWATERING	QUALITY				0 × 1	100'		
1	SS'.56 DOMESTIC	5 🕱 C	OMMERCIÁL UNICIPAL					£.6.3	6. 0	person frequency	
WATER USE	2 STOCK 3 RRIGATION 4 NOUSTRIAL	7 🗆 P	UNICIPAL, UBLIC SUPPLY DOLING OR AIR COND	ITTONING					÷4.		
	OTHER		, D NO	T USED					Λ.		
METHOD		ENTIONAL)	BORING DIAMOND					Highbury	HUE.	^-	.000
CONSTRUCT	ION ROTARY (REVEI		B ☐ JETTING • ☐ DRIVING ☐ DIGGING	OTHER		DRULLERS REMAR				67	992
NAME OF WEL	L CONTRACTOR			L CONTRAC	_	> DATA SOURCE	5.0	CONTRACTOR	DATE RECEIVE		63-64 40
1	JONES DRILLIN	NG LTD.		009	_	DATE OF INSP	ECTION	3 0 0 9	MAY	0219	91
R. R.	#3 Thomndale,	Ontari	o NOM 2PO			SE					
MURRA!	S. JONES		T T	L TECHNIC	ER						
SIGNATURE	OF TECHNICIAN/CONTRACTO	nes	SUBMISSION DATE		91	OFFICE			CS	S.S8	
MINISTRY	OF THE ENVIRO	NMENT (F	ORM NO. 0506	(11/86) FOT

Print only in space Mark correct box v		, where applic	cable.	11	4	1144	96	d .	Con	אכ	
County or District			To	wnship/Borough	/City/Town/Vill	age		Con	block tract surve	ey, etc.	Lot 25-27
Owner's surname	iddleser	First Name	Ad	Jondon dress					Date completed	20 (<u>8</u> 26 00
BWH Holdi	ngs Inc.	Zone	Easting 24	O4 High	ury Ave	RC R Ele	1 Arva	Basin		20 C	month yea
21		W 10	1 1 1 1	LJ L1	1111	25 26		31			يتتبل
		LOG (OF OVERBU	RDEN AND I	BEDROCK N	IATERIALS (see instruc	tions)		T 5	
General colour	Most common	n material		Other mater	als		Gener	al descripti	ion	From	epth - feet To
	PREVIOUSLY	Y DRILLE	D.							0	49
<u> </u>				Nation .							
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31			عالب		ىيا لىيا		ـــا لــــــ		ــالــــــــــــــــــــــــــــــــــ	444	البلبا
32	15 21	51	CACING	& OPEN HO	LI L		54	of opening	31-33 Diameter	34-38	ength 39-40
Water found	Kind of water	Inside	е	Wall	De	pth - feet		lo.)	J. J	inches	feet
at - feet	Erech 3 🗆 Sulphur	14 inche	s	inches	From	To 13-16	Materi	al and type		Depth at	top of screen
15-18	Salty 6 Gas		2 ☐ Galvar 3 ☐ Concre	ete			8			<u> </u>	feet
2 -		17:1	4 Open i			20-23	61		ING & SEALING		
20-23 1 2	Fresh 3 Sulphur Minerals	24	1 ☐ Steel 2 ☐ Galvar 3 ☐ Concre	nized ete			Depth se		space Material and type (C	Abando	
25-28 1 🔲	Fresh ³ Sulphur	29	4 ☐ Open I	hole			From 10-13	14-17	_		,, 55,115,1110, 5101,
2 🗆 :	Salty 6 Gas		2 Galvar	nized		27-30	18-21	47 22-25 49	Bentonite		
2 🗆	Fresh 3 Sulphur Salty 6 Gas	، " [" [3 Concre 4 Open I	hole			26-29	30-33	"Pea Grave	1	
Pumping test met		no rate	1-14 Duration o	of pumping							
/1 □ Pump 2 □	Bailer		РМ	Hours M	ns II8	In diagra			OF WELL ces of well from	road and	l lot line.
_ Static level and	or pumping	er levels during	1 Pumping	² ☐ Reco			north by arr				
SIL 19-21 SIL 19-21 SIL 19-21 If flowing give rate	22:24 15 minu	tes 30 minute	45 minutes		- 11 7						_
If flowing give rate	feet 38-41 Pump in	feet ntake set at	Water at e	feet end of test	feet 1				HIGHBURY	AVE	
Recommended pur	GPM Recomm	mended 4	feet C		ty 16 49						
☐ Shallow ☐	Deep pump so	etting	pump rat	te	БРМ						
50-53	OF WELL				∃ I					4500	AY RD.
FINAL STATUS	UF WELL 5 ☐ Aba	andoned, insufficie	ent supply 9 🗆	Unfinished Replacement wel	Ш					TEDW	M/KD
3 ☐ Test hole 4 ☐ Recharge w	7 🙀 Aba	indoned (Other)	anty U	пориссители чег	Ш				× 2404		
WATER USE		55-56									
Domestic Stock	5 ☐ Con 6 ☐ Mur	nmercial nicipal		Not use Other							
3 ☐ Irrigation 4 ☐ Industrial	7 ☐ Pub 8 ☐ Coo	olic supply oling & air conditio	oning								
METHOD OF CO	ONSTRUCTION :	57						ı			
1	5 ☐ Air p	percussion ina	10	Driving Digging	Ш						
3 ☐ Rotary (reve	rse) ⁷ ☐ Diaz ⁸ ☐ Jetti	mond	" 🗆	Other						217	7711
							. Control		so se Pote reas		
Name of Well Contract Mervin Jone		o I.td	30x	ontractor's Licenc	¥	ata ource	Se Contractor	000	3 Sa 62 Date rece		2000
Address				,		ate of inspection		Inspector	, , , , , , ,		
22264 Fair Name of Well Technici	view Rd, T	horndale	Ontar	chnicians Licence	PR. 💆	emarks		_L			
Murray S Signature of Technicia	Jones		TO	068 ssion date	MINISTRY					CSS	.ESO
Signature of Technicia	2 1	طد	d 20	96 09							
	m - 7 - 7 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1		RONMEN							0506 (11	1/98) Front Form

	Ministry of he Environr		Number (Plac			Regulation 90	3 Ontari	Well R	
Instructions for Completin For use in the Province All Sections must be con Questions regarding com All metre measurement Please print clearly in blu	of Ontario on pleted in full pleting this is shall be r	II to avoid delays application can be eported to 1/10 th	ent is a permain processing directed to	g. Further the Water	al document. Finstructions an Well Manage	d explanations are ava ment Coordinator at Ministry Us	ailable o 416-23	n the back of 5-6203.	this form.
Address of vivel Location (County) Address of vivel Location (County) RR#/Street Number/Name 2/554 GPS Reading NaD Zoon	ivry Rå	cipality) Northi		/nship /nship Da ll / Jnit Make/M	illage		artment/E	Concession Block/Tract et	С.
8 3 1 1 1 Log of Overburden and Be	1418101	B63 417	681 1717	Garr			erentiated,		aged
General Colour Most common		Other Mat			Genera	al Description		Depth From I	Metres To
GRET CLAY	Mark 1							31	15'
BONN SAND		rocks	×	500	dy	4.161		0'	3'
	7.47		<u>* </u>						
						21			
	4 4		e de la composition della comp						
Salude	- 97	5 10000	, 11						
cusiu	0	Judas	2	1	Part Part 1				
Hole Diameter	16.00	Const	ruction Reco	rd			t of Wel		
Depth Metres Diameter From To Centimetres	Inside diam	Material	Wall thickness	Depth	Metres	Pumping test method	Draw Time Wa		ecovery Water Level
0' 4"	centimetres		centimetres	From	То	Pump intake set at -	min M Static	Metres min	Metres
•		Steel Fibreglass	Casing			(metres) Pumping rate -	Level	1.12	/
Water Record	21/2	Plastic Concrete			3'	(litres/min) Duration of pumping			
Water found / Kind of Water &		Galvanized Steel Fibreglass		, k,	1,000	hrs + min	2	2	W. sanda
m Fresh Sulphur Gas Salty Minerals		Plastic Concrete				Final water level end of pumping metres	3	3	A To The Services
Other:		Galvanized Steel Fibreglass				Recommended pump	4	4	
m Fresh Sulphur Gas Salty Minerals		Plastic Concrete				Recommended pump	5	5	
Other:		Galvanized	C		1 2	depthmetres			
m Fresh Sulphur Gas Salty Minerals	Outside	Steel Fibreglass	Slot No.			rate. (litres/min)	10	10	
Other:After test of well yield, water was	diam	Plastic Concrete	Old: No.	3'	15	If flowing give rate - (litres/min)	20	20 25	
Clear and sediment free	2' 🖺	Galvanized			** ' · · · · ·	If pumping discontin- ued, give reason.	30	30	
Other, specify			sing or Scree	en			40 50	40 50	
Chlorinated Yes No		Open hole					60	60	
Plugging and Sea	ling Record	Annular s	space Aba	ndonment Placed	In diagram below	Location o	of Well	lot line, and bui	Idina
A 21 -		1 425 10 11	(cubic r	netres)	Indicate north by	v show distances of well fro arrow. トルハウラル	7	iot line, and but	idiig.
3 15 600		ND S	15	7, 1			.111		
						40	311	γ ₂ γ ₂	
						1	111	- 1	
M	ethod of Co	nstruction		·	Ja 1835			\$	
Cable Tool Rotary (conventional) Air percu Rotary (reverse) Boring	ssion	☐ Diamond ☐ Jetting ☐ Driving		Digging Other		PETRO			
Domestic Industrial	Water L	Public Supply		Other			- A	W.	
Stock Commerc		Not used Cooling & air of			Audit No.	40007 Date	e Well Co	mpleted	
	Final Status	of Well	1.0			43301		2007 0	MM DD
☐ Water Supply ☐ Recharge well ☐ Observation well ☐ Abandoned, in	nsufficient supp		Abandon	ed, (Other)	Was the well ow package delivere		e Delivered	2007	5/1
		Replacement ician Information	1 1			/ Ministry Use			
Vayme of Well Contractor 501L	EST	(272) Weil	Contractor's Lic	ence No.	Data Source	Con	tractor	190	*
Business Address (street name, numbe	r, city etc.)	OM Alor	1A)		Date Received	IUN 1 4 M 2007 Date	e of Inspec	tion yyyy	MM DD
vame of Well Technician (last pame, fir	st name)	Well	Technicien's Lic	ence No.	Remarks		Record N	Number	
Signature of Technician/Contractor	M//	Date \$	Submitted yary	MM 29//	Y .		*		
)506E (09/03)	Contrac	tor's Copy Mini	stry's Copy 🔀	Well Own	L er's Copy ☐ ^{®®}	Cette fo	rmule es	st disponible e	n français

Ontario Ministry of the Environment and Climate Change Measurements recorded in: Metric Imperial	Tag No. (Place Sticker	·		Vell Record Vater Resources Act
Address of Well Location (Street Number/Name) 21559 High bury Ave. N.	Township dilese	x Centre Lot 9	Concessi	on
County/District/Municipality Middlesex	City/Town/Village		Province	Postal Code
UTM Coordinates Zone , Easting , Northing	Municipal Plan and Sub	lot Number	Ontario Other	NOMILLO
NAD 8 3 1 74 8 0 5 0 3 4 7 6 8 1 7 6 Overburden and Bedrock Materials/Abandonment Sealing Rec	cord (see instructions on th	e back of this form)	Section 1997	
General Colour Most Common Material O	ther Materials	General Description	on	Depth (m/ft) From To
No Previou	us Repo	rt Found.		
42" Dug	well	المام المام	1	0 1
		Chèse sind	04	0 1
		Chips and Note Plug	. VWS	7 8.5
		Washed Pea	stone	8.5 10
Annular Space		Results of V	Vell Yield Testing	
Pepth Set at (m/it) Type of Scalant Used From To (Material and Type)	Volume Blaced	After test of well yield, water was:	Draw Down	Recovery /
		Onger, specify	(min) (m/it)	(min) (guti)
		If pumping discontinued, give reason	Level	+/ $-$
		Pump intake set at (m/ft)	2	2
		Dumning ento ((mia (CON)	3	3
Method of Construction Well U Cable Tool		Pumping rate (Vmin / GPM)	4	4
☐ Rotary (Conventional) ☐ Jetting ☐ Domestic ☐ Munits ☐ Rotary (Reverse) ☐ Driving ☐ Livestock ☐ Test H	Ral Dewatering	Duration of pumping hrs + min	5	5
	& Air Conditioning	Final water level end of pumping (mg	10	10
Tother, specify Other, specify		If flowing give rate (t/min / GPM)	15	15
Construction Record - Casing Inside Open Hole OR Material Wall Depth (m/ft) Diamster (Galvaniert, Fibreniass Thickness	Status of Well Water Supply	Recommended pump depth (m/ft)	20	20
Diamster (Galvanized, Fibreglass, Comfress (cm/in) Concrete, Plastic, Steel) (cm/in) From To	Replacement Well Test Hole		25	25
	Recharge Well Dewatering Well	Recommended pump rate (Vmin / GPM)	30	80
	Observation and/or Monitoring Hole	Well production (Vmin / GPM)	40	40
	Alteration (Construction)	Disiniscled?	50	50
Construction Record - Screen	Abandoned, Insufficient Supply	₩Yes No	60 elt Location	60
Outside Diameter Material Slot No. Depth (m/ft)	Abandoned, Poor Water Quality Abandoned, other,	Please provide a map below following		oack.
(cm/in) (Flastic, Gatvanizad, Steal) From To	specify			
	Other, specify			
Water Details	DECOMP lole Diameter			
	th (m/h) Diameter To (cm/in)			
Nater found at Depth Kind of Water: Fresh Untested				
(m/tt) Gas Other epecify Nater found at Depth Kind of Water: Fresh Untested				
(m/ft) Gas Other, specify				
Well Contractor and Well Technician Informatiusiness Name of Well Contractor We				
	Contractor's Licence No. 	Comments:		
21937 Highbury Ave N A	ricipanty	Comments:		
ON NIDMILICIO STAINTONS @XD OI	net.com	Well owner's Date Package Delivere	d Minier	try Use Only
ius. Telephone No. (inc. area code) Name of Well Technician (Last Name)	Circt Mome)	information package Y Y Y Y M M	Apdit No ::	242464
51196579333579 Stainton Bre	e Submitted	Yes Date Work Completed		0 0 0 004B
508E (2014/11)	Y Y Y M M D D Ministry's Copy	□ No 2 01 70 5		6 6 2 2019 Printer for Ontario, 2014

Wastewater Assessment Page 36

Appendix E

Wastewater Treatment System Assumptions & Sizing

Wastewater Assessment Page 37

WASTEWATER TREATMENT SYSTEM ASSUMPTIONS AND SIZING

MIDDLESEX CENTRE REQUIRES SUFFICIENT SPACE FOR A CONTINGENCY BED DURING LOT CREATION. HENCE, TERTIARY PRETREATMENT SYSTEMS ARE REQUIRED FOR PROPOSED LOT SIZES.

ASSUMED HOUSE CHARACTERISTICS

FIXTURE UNITS - SUM	MARY No.	LOAD	TOTAL
TT CIVI	110.	LOAD	TOTAL
1.FULL BATHROOM INDIVIDUAL ITEMS :	4	6	24
2. ANY TYPE OF BATH	2	1.5	3.0
3. FLUSH TANK TOILETS	1	4	4
4a.SHOWER(1 HEAD)	0	1.5	0
4b.SHOWER(3 HEAD)	0	4.5	0
5.FLOOR DRAIN	1	2 - 4	3
6.LAVATORY (DOMESTIC)	1	1.5	1.5
7.BIDET	0	1	0
8. KITCHEN SINK	1	1.5	1.5
DISHWASHER (to sink trap)	1	0	0
10. LAUNDRY TUB	1	1.5	1.5
11. CLOTHES WASHER	1	1.5	1.5
12. DRINKING FOUNTAIN	0	0.5	0
13. GARBAGE GRINDER	0	3	0
TOTAL UNITS			40.0
NO. OF BEDROOMS:		4	
TOTAL LIVING AREA:		300 m ²	

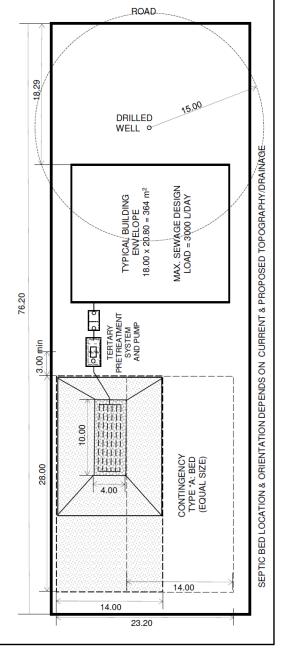
WASTE SYSTEM - DESIGN CAPACITY

BASE LOAD (4 BEDROOM): 2000 1. F. U. OPTION (40 - 20) X 50 : 1000 2. L. A. OPTION (300- 200)/10 X 100: 1000 ADD HIGHER OF ITEM 1 OR 2

DESIGN LOAD = 3000 L/DAY

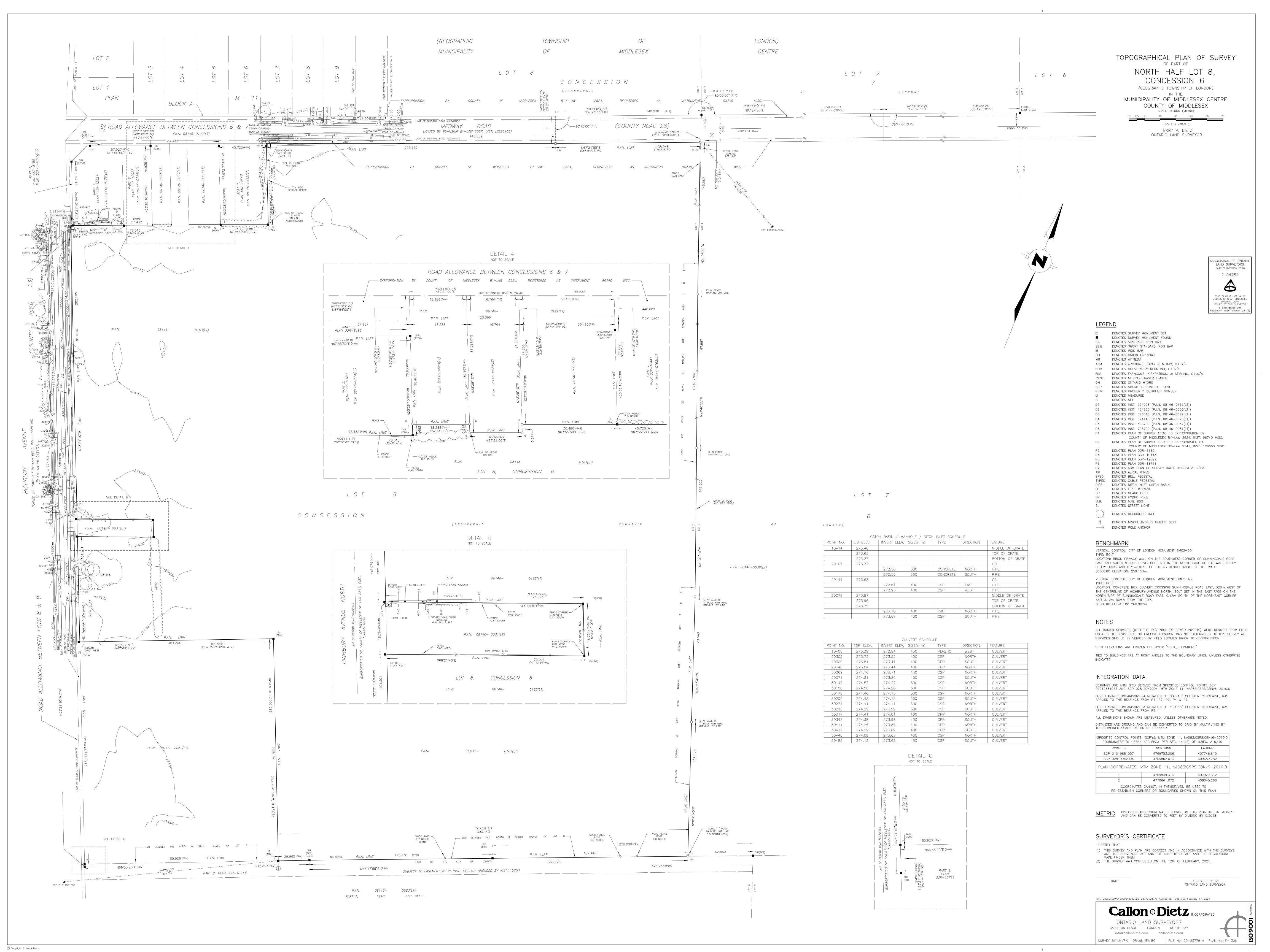
CAN/BNQ 3680-600 TYPE "A" BED SEPTIC SYSTEM DESIGN CALCULATIONS AND DIMENSIONS

- 1. DESIGN LOAD = 3000 L/DAY (SEE "DESIGN CAPACITY")
- 2. TERTIARY PRETREATMENT UNIT REQUIRED.
- 2. MIN. STONE BED AREA = 3000 / 75 = 40.0 m² SPECIFIED: 10.00 X 4.00 = 40.0 m²
- 3. DISTRIBUTION PIPE: 6 RUNS EACH 8.80 m LONG @ 56 cm ON CENTRES; ALL PIPES 60 cm FROM EDGES OF STONE.
- 4. MIN. TOTAL SAND CONTACT AREA = QT/400 = 375 m^2 TOTAL SPECIFIED AREA INCL MANTLE = $20.90 \, \text{X} \, 18.00 = 376 \, \text{m}^2$
- 5. IMPORTED SAND : T = 6 to 8 min/cm



Wastewater Assessment Page 38







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KITCHENER LOCATION

1415 Huron Rd., Unit 225 Kitchener, ON N2R 0L3 P: 519-725-8093

PLANNING · CIVIL · STRUCTURAL · MECHANICAL · ELECTRICAL **SWM Calculations**

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DATE: March 18, 2021 SBM-21-0716 JOB NO.:

Brock Development Group Client: **Proposed Residential Development Project:** Medway Road, Ballymote, Ontario Location:

CITY OF LONDON-3 CHICAGO RAINFALL DISTRIBUTION PARAMETERS*

Patura Pariod (vaars)	A,B,C Parameters				
Return Period (years)	Α	В	С		
25mm	538.850	6.331	0.809		
2	1290.000	8.500	0.860		
5	1183.740	7.641	0.838		
10	1574.382	9.025	0.860		
25	2019.372	9.824	0.875		
50	2270.665	9.984	0.876		
100	2619.363	10.500	0.884		
250	3048.220	10.030	0.888		

Runoff Coefficients from Section 4.8.3 of Municipality of Middlesex Centre Infrastructure Design Standards (IDS)

Parks, open space and playgrounds = 0.2 Standard Single Family Residential = 0.5

A201, A202 & A203 - LOT 1 (LARGEST AREA OF LOTS 1, 2 & 3)

LOT 1 - A201

PRE-DEVELOPMENT CONDITIONS (A201)

	Area (m²)	
Total Site Area:	2241.28	
Runoff Coefficient C =	0.20	
5-Year Pre-Development Flows		
C-value =	0.20	
**Time of concentration t_c =	19	min
Intensity, i (@ t_c) =	74.60	mm/hr
Pre-Development Flow, $Q_r = 2.78 C^*i^A =$	9.30	I/s

5-Year Post-Development Flows

3-real rost-bevelopilient riows		
C-value =	0.50	
**Time of concentration t_c =	19	min
Intensity, i (@ t_c) =	75.62	mm/hr
Post-Development Flow, $Q_r = 2.78 \text{ °C*i*A} =$	23.56	I/s
·		_

POST-DEVELOPMENT CONDITIONS (A201)

	Area (m²)
Total Site Area:	2241.28
$C_{eq} = Sum(A*C)/Sum(A) =$	0.50

100-Year Pre-Development Flows

C-value =	0.20	
**Time of concentration t_c =	19	min
Intensity, i (@ t_c) =	131.48	mm/hr
Pre-Development Flow, $Q_r = 2.78 * C*i * A = 0.000$	16.38	I/s

100-Year Post-Development Flows

•		
C-value =	0.50	
**Time of concentration t_c =	19	min
Intensity, i (@ t_c) =	153.12	mm/h
Post-Development Flow, $Q_r = 2.78 C^*i^A =$	47.70	l/s
·		

^{**}Time of concentration from Section 4.8.2 - Municipality of Middlesex Centre - Infrastructure Design Standards

SOAK-AWAY PIT DETAILS

Stone Depth	1.0	m
Stone Width=	3.0	m
Stone Length=	26.5	m
19mm Clear Stone Void Ratio=	0.35	İ
Infiltration Rate=	1.33E-06	m/
Storage (Total)=	27.825	m^3
Contact Area to Soil (Trench Side Walls Only)=	138.50	m^2

Design Infiltration rate calculation Coeffic

cient of permeability (K)=	0.1	cm/sec
Percolation time (T)=	50	mins/cr
Infiltration rate (1/T)	12	mm/hr
Safety correction factor	2.5	
Design Infiltration rate	1.33E-06	m/s

(Conservative Assumption)

(Table C3 TRCA)

INFILTRATION RATE CALCULATIONS

A=	138.500	m² Contact Area to Soi
l=	1.33E-06	m/s

Δ*Ι	1 85F-04	m ³ /s

5 Year Design S	torm Event						
		Inflow, Q _i	Volume In	Allowable Surface Outflow	Exfiltration	Total Volume Out	Difference/
Duration	Intensity "i"	2.78*C*i*A	Qt*t*60/1000	Q_{o}	Volume	Q _o *t*60/1000	Storage
(min.)	(mm/hr)	(I/s)	(m ³)	(I/s)	(m³)	(m ³)	(m³)
19	75.62	23.56	26.86	9.30	0.21	10.81	16.05
23	67.26	20.95	28.92	9.30	0.25	13.08	15.83
30	56.60	17.63	31.74	9.30	0.33	17.07	14.68
60	34.64	10.79	38.85	9.30	0.66	34.13	4.71
120	20.34	6.34	45.63	9.30	1.33	68.27	0.00
180	14.73	4.59	49.56	9.30	1.99	102.40	0.00
Reference: Soil Report for Wastewater Ser	vicing by BOS Engineering & Environm	nental Services Inc. The percolatio	n time (T) of the soil on-s	ite is greater than 50 min/cm		Max. Storage Volume (m³) =	16.05

Reference: Soil Report for Wastewater Servicing by BOS Engineering & Environmental Services Inc. The percolation time (T) of the soil on-site is greater than 50 min/cm

100 Year Desig	n Storm Event						
		Inflow, Q _i	Volume In	Allowable Surface Outflow	Exfiltration	Total Volume Out	Difference/
Duration	Intensity "i"	2.78*C*i*A	Qt*t*60/1000	Q_{\circ}	Volume	Q _o *t*60/1000	Storage
(min.)	(mm/hr)	(I/s)	(m ³)	(I/s)	(m ³)	(m³)	(m ³)
19	131.48	40.96	46.70	16.38	0.21	18.89	27.81
23	117.50	36.61	50.52	16.38	0.25	22.87	27.65
30	99.36	30.95	55.72	16.38	0.33	29.83	25.89
60	60.87	18.96	68.27	16.38	0.66	59.65	8.62
120	35.32	11.00	79.22	16.38	1.33	119.30	0.00
180	25.28	7.88	85.06	16.38	1.99	178.95	0.00
ce: Soil Report for Wastewater S	ervicing by BOS Engineering & Environm	ental Services Inc. The percolat	ion time (T) of the soil on-	site is greater than 50 min/cm		Max. Storage Volume (m ³) =	27.81

Total Storage Available within infiltration trench (m³) =	27.83
Required 5 Year Storage (m³) =	16.05
Required 100 Year Storage (m³) =	27.81

	Drawdown time for soak-away			
5 year storm events	24.14	hrs		
100 year storm events	41.83	hrs		

^{*} Intensity i=A/(t+B)^C (mm/hr)

^{*} Section 6.2 of Municipality of Middlesex Centre IDS, City of London a,b, & c design parameters are used.



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PLANNING · CIVIL · STRUCTURAL · MECHANICAL · ELECTRICAL **SWM Calculations**

> March 18, 2021 SBM-21-0716

Brock Development Group Client: **Project: Proposed Residential Development** Medway Road, Ballymote, Ontario Location:

CITY OF LONDON-3 CHICAGO RAINFALL DISTRIBUTION PARAMETERS*

Return Period (years)		A,B,C Parameters	
	Α	В	С
25mm	538.850	6.331	0.809
2	1290.000	8.500	0.860
5	1183.740	7.641	0.838
10	1574.382	9.025	0.860
25	2019.372	9.824	0.875
50	2270.665	9.984	0.876
100	2619.363	10.500	0.884
250	3048.220	10.030	0.888
+! A //+ - D\AC			

Runoff Coefficients from Section 4.8.3 of Municipality of Middlesex Centre Infrastructure Design Standards (IDS)

min

mm/hr

Parks, open space and playgrounds = 0.2 Standard Single Family Residential = 0.5

A204 - LOT 4

PRE-DEVELOPMENT CONDITIONS (A204)

DATE:

JOB NO.:

	Area (m²)	
Total Site Area:	2170.03	
Runoff Coefficient C =	0.20	
5-Year Pre-Development Flows		
C-value =	0.20	
**Time of concentration t_c =	19	min
Intensity, i (@ t_c) =	74.60	mm/hr
Pre-Development Flow, $Q_r = 2.78 \text{ °C*i*A} = $	9.00	I/s

5-Year Post-Development Flows					
C-value =	0.50				
**Time of concentration t_c =	19	min			
Intensity, i (@ t_c) =	75.62	mm/hr			
Post-Development Flow, $Q_r = 2.78 \text{ °C*i*A} = $	22.81	I/s			

POST-DEVELOPMENT CONDITIONS (A204)

	Area (m²)	
Total Site Area:	2170.03	
$C_{eq} = Sum(A*C)/Sum(A) =$	0.50	
100-Year Pre-Developmen	t Flows	
C-value =	0.20	
**Time of concentration t _c =	19	min

Intensity, i (@ t_c) =	131.48	mm/h
Pre-Development Flow, $Q_r = 2.78 C^*i^*A =$	15.86	I/s
		_

100-Year Post-Development Flows 0.50 C-value = 19 **Time of concentration t_c=

Intensity, i (@ t_c) =

Post-Development Flow, $Q_r = 2.78 C^*i^*A =$

SOAK-AWAY PIT DETAILS

Stone Depth	1.0	m
Stone Width=	3.0	m
Stone Length=	25.7	m
19mm Clear Stone Void Ratio=	0.35	
Infiltration Rate=	1.33E-06	m/s
Storage (Total)=	26.985	m ³
Contact Area to Soil (Trench Side Walls Only)=	134.50	m^2
		-

Design Infiltration rate calculation
Coefficient of permeability (K)=
Percolation time (T)=
Infiltration rate (1/T)

Safety correction factor Design Infiltration rate

153.12

46.19

0.1 50	cm/sec mins/cm
12	mm/hr
2.5	
1.33E-06	m/s

Max. Storage Volume (m³)

Max. Storage Volume (m³)

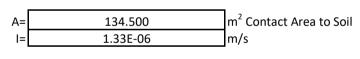
(Conservative Assumption)

(Table C3 TRCA)

15.54

26.92

INFILTRATION RATE CALCULATIONS



Reference: Soil Report for Wastewater Servicing by BOS Engineering & Environmental Services Inc. The percolation time (T) of the soil on-site is greater than 50 min/cm

Reference: Soil Report for Wastewater Servicing by BOS Engineering & Environmental Services Inc. The percolation time (T) of the soil on-site is greater than 50 min/cm

^{1.79}F-04

A*I[1./9E-04	Jm [*] /s					
5 Year Design Sto	rm Event						
		Inflow, Q _i	Volume In	Allowable Surface Outflow	Exfiltration	Total Volume Out	Difference/
	Intensity "i"	2.78*C*i*A	Qt*t*60/1000	Q_{o}	Volume	Q _o *t*60/1000	Storage
	(mm/hr)	(I/s)	(m ³)	(I/s)	(m ³)	(m ³)	(m³)
	75.62	22.81	26.00	9.00	0.20	10.47	15.54
	67.26	20.29	28.00	9.00	0.25	12.67	15.33
	56.60	17.07	30.73	9.00	0.32	16.53	14.21
	34.64	10.45	37.61	9.00	0.65	33.05	4.56
	20.34	6.14	44.18	9.00	1.29	66.10	0.00
	14.73	4.44	47.99	9.00	1.94	99.15	0.00

100 Year Design S	torm Event						
		Inflow, Q _i	Volume In	Allowable Surface Outflow	Exfiltration	Total Volume Out	Difference/
Duration	Intensity "i"	2.78*C*i*A	Qt*t*60/1000	Q_{o}	Volume	Q _o *t*60/1000	Storage
(min.)	(mm/hr)	(l/s)	(m ³)	(I/s)	(m³)	(m³)	(m³)
19	131.48	39.66	45.21	15.86	0.20	18.29	26.92
23	117.50	35.44	48.91	15.86	0.25	22.14	26.77
30	99.36	29.97	53.95	15.86	0.32	28.88	25.07
60	60.87	18.36	66.10	15.86	0.65	57.76	8.34
120	35.32	10.65	76.70	15.86	1.29	115.51	0.00
180	25.28	7.63	82.35	15.86	1.94	173.27	0.00

Total Storage Available within infiltration trench (m³) =	26.99	Soakaway pit drawdown tir	ne at 0.18L/s infiltra	ation rate	Surface pond drawdown time at 0.18L/s infiltration rate
Required 5 Year Storage (m³) =	15.54	5 year storm events	24.07	hrs	
Required 100 Year Storage (m³) =	26.92	100 year storm events	41.70	hrs	All storm events 11.00

^{*} Intensity i=A/(t+B)^C (mm/hr) * Section 6.2 of Municipality of Middlesex Centre IDS, City of London a,b, & c design parameters are used.

^{**}Time of concentration from Section 4.8.2 - Municipality of Middlesex Centre - Infrastructure Design Standards



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DATE: March 18, 2021
JOB NO.: SBM-21-0716

Client: Project: Location:

Brock Development Group
Proposed Residential Development
Medway Road, Ballymote, Ontario

CITY OF LONDON-3 CHICAGO RAINFALL DISTRIBUTION PARAMETERS*

Return Period (years)		A,B,C Parameters	
	Α	В	С
25mm	538.850	6.331	0.809
2	1290.000	8.500	0.860
5	1183.740	7.641	0.838
10	1574.382	9.025	0.860
25	2019.372	9.824	0.875
50	2270.665	9.984	0.876
100	2619.363	10.500	0.884
250	3048.220	10.030	0.888

Infrastructure Design Standards (IDS)

Runoff Coefficients from Section 4.8.3 of Municipality of Middlesex Centre

Parks, open space and playgrounds = 0.2 Standard Single Family Residential = 0.5

A205 to A214 & A217 to A219 - LOT 6 (LARGEST AREA OF LOTS 5 to 14 & 17 to 19)

LOT 6 - A206

PRE-DEVELOPMENT CONDITIONS (A206)

Post-Development Flow, $Q_r = 2.78 C^*i^A =$

Total Site Area:	Area (m²) 1867.89	
Runoff Coefficient C =	0.20	
5-Year Pre-Development Flows		
C-value =	0.20	
**Time of concentration t_c =	19	min
Intensity, i (@ t_c) =	74.60	mm/hr
Pre-Development Flow, $Q_r = 2.78 \text{ °C*i*A} = $	7.75	I/s
5-Year Post-Development Flows		
C-value =	0.50	
**Time of concentration t_c =	19	min
Intensity, i (@ t_c) =	75.62	mm/hr

POST-DEVELOPMENT CONDITIONS (A206)

	Area (m²)
Total Site Area:	1867.89
$C_{eq} = Sum(A*C)/Sum(A) =$	0.50

100-Year Pre-Development Flows

100-Year Post-Development Flows

19.63

SOAK-AWAY PIT DETAILS

Stone Depth	1.0 r	n
Stone Width=	3.0 r	n
Stone Length=	22.1 r	n
19mm Clear Stone Void Ratio=	0.35	
Infiltration Rate=	1.33E-06	n,
Storage (Total)=	23.205 r	'n
Contact Area to Soil (Trench Side Walls Only)=	116.50 r	'n

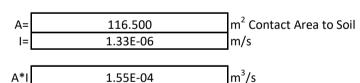
<u>Design Infiltration rate calculation</u> Coefficient of permeability (K)=

icient of permeability (K)=	0.1	cm/sec
Percolation time (T)=	50	mins/cm
Infiltration rate (1/T)	12	mm/hr
Safety correction factor	2.5	
Design Infiltration rate	1.33E-06	m/s

(Conservative Assumption)

(Table C3 TRCA)

INFILTRATION RATE CALCULATIONS



Reference: Soil Report for Wastewater Servicing by BOS Engineering & Environmental Services Inc. The percolation time (T) of the soil on-site is greater than 50 min/cm

5 Year Design St	orm Event						
		Inflow, Q _i	Volume In	Allowable Surface Outflow	Exfiltration	Total Volume Out	Difference/
Duration	Intensity "i"	2.78*C*i*A	Qt*t*60/1000	Q_{o}	Volume	Q _o *t*60/1000	Storage
(min.)	(mm/hr)	(I/s)	(m ³)	(I/s)	(m³)	(m ³)	(m³)
19	75.62	19.63	22.38	7.75	0.18	9.01	13.37
23	67.26	17.46	24.10	7.75	0.21	10.91	13.19
30	56.60	14.70	26.45	7.75	0.28	14.23	12.23
60	34.64	8.99	32.37	7.75	0.56	28.45	3.92
120	20.34	5.28	38.03	7.75	1.12	56.90	0.00
180	14.73	3.82	41.30	7.75	1.68	85.36	0.00

100 Year Design	Storm Event						
		Inflow, Q _i	Volume In	Allowable Surface Outflow	Exfiltration	Total Volume Out	Difference/
Duration	Intensity "i"	2.78*C*i*A	Qt*t*60/1000	Q_{o}	Volume	Q _o *t*60/1000	Storage
(min.)	(mm/hr)	(I/s)	(m ³)	(I/s)	(m³)	(m³)	(m ³)
19	131.48	34.14	38.92	13.66	0.18	15.74	23.17
23	117.50	30.51	42.10	13.66	0.21	19.06	23.04
30	99.36	25.80	46.43	13.66	0.28	24.86	21.58
60	60.87	15.80	56.89	13.66	0.56	49.72	7.18
120	35.32	9.17	66.02	13.66	1.12	99.44	0.00
180	25.28	6.56	70.89	13.66	1.68	149.15	0.00
Reference: Soil Report for Wastewater Ser	ference: Soil Report for Wastewater Servicing by BOS Engineering & Environmental Services Inc. The percolation time (T) of the soil on-site is greater than 50 min/cm Max. Storage Volume (m³) =						23.17

Total Storage Available within infiltration trench (m³) =	23.21
Required 5 Year Storage (m³) =	13.37
Required 100 Year Storage (m³) =	23.17

	Drawdown time for soak-a	way pi
5 year storm events	23.91	hrs
100 year storm events	41.44	hrs

Max. Storage Volume (m³)

13.37

^{*} Intensity i=A/(t+B)^C (mm/hr)

^{*} Section 6.2 of Municipality of Middlesex Centre IDS, City of London a,b, & c design parameters are used.

 $^{{\}tt **Time~of~concentration~from~Section~4.8.2~-~Municipality~of~Middlesex~Centre~-~Infrastructure~Design~Standards}$



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(Conservative Assumption)

(Table C3 TRCA)

DATE: March 18, 2021 SBM-21-0716 JOB NO.:

Brock Development Group Client: **Project: Proposed Residential Development** Medway Road, Ballymote, Ontario Location:

CITY OF LONDON-3 CHICAGO RAINFALL DISTRIBUTION PARAMETERS*

Return Period (years)		A,B,C Parameters	
Return Period (years)	Α	В	С
25mm	538.850	6.331	0.809
2	1290.000	8.500	0.860
5	1183.740	7.641	0.838
10	1574.382	9.025	0.860
25	2019.372	9.824	0.875
50	2270.665	9.984	0.876
100	2619.363	10.500	0.884
250	3048.220	10.030	0.888

Runoff Coefficients from Section 4.8.3 of Municipality of Middlesex Centre Infrastructure Design Standards (IDS)

Parks, open space and playgrounds = 0.2 Standard Single Family Residential = 0.5

* Intensity i=A/(t+B)^C (mm/hr)

* Section 6.2 of Municipality of Middlesex Centre IDS, City of London a,b, & c design parameters are used.

A215 - LOT 15

POST-DEVELOPMENT CONDITIONS (A215)

Total Site Area:

100-Year Pre-Development Flows

Intensity, i (@ t_c) =

100-Year Post-Development Flows

C-value =

C-value =

 $C_{eq} = Sum(A*C)/Sum(A) =$

**Time of concentration t_c =

Pre-Development Flow, $Q_r = 2.78 C^*i^*A =$

PRE-DEVELOPMENT CONDITIONS (A215)

	Area (m²)	
Total Site Area:	1950.43	
Runoff Coefficient C =	0.20	
5-Year Pre-Development Flows		
C-value =	0.20	
**Time of concentration $t_c =$	19	min
Intensity, i (@ t_c) =	74.60	mm/hr
Pre-Development Flow, $Q_r = 2.78 C^*i A = $	8.09	I/s
5-Year Post-Development Flows		

C-value =

**Time of concentration t_c =	19	min	**Time of concentration t_c =
Intensity, i (@ t_c) =	75.62	mm/hr	Intensity, i (@ t_c) =
Post-Development Flow, $Q_r = 2.78 C^*i^*A =$	20.50	I/s	Post-Development Flow, $Q_r = 2.78 * C * i * A = $

0.50

SOAK-AWAY PIT DETAILS

Stone Depth	1.0	m
Stone Width=	3.0	m
Stone Length=	23.1	m
19mm Clear Stone Void Ratio=	0.35	
Infiltration Rate=		m/s
Storage (Total)=	24.255	m^3
Contact Area to Soil (Trench Side Walls Only)=	121.50	m^2
E CALCULATIONS		

19 153.12 41.51

Area (m²) 1950.43

0.50

0.20

19

131.48

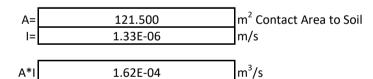
14.26

0.50

Design Infiltration rate calculation			
Coefficient of permeability (K)=	0.1	cm/sec	
Percolation time (T)=	50	mins/cm	
Infiltration rate (1/T)	12	mm/hr	
Safety correction factor	2.5		
Design Infiltration rate	1.33E-06	m/s	

min

INFILTRATION RATE CALCULATIONS



5 Year Desig	n Storm Event						
		Inflow, Q _i	Volume In	Allowable Surface Outflow	Exfiltration	Total Volume Out	Difference/
Duration	Intensity "i"	2.78*C*i*A	Qt*t*60/1000	Q_{o}	Volume	Q _o *t*60/1000	Storage
(min.)	(mm/hr)	(I/s)	(m ³)	(I/s)	(m³)	(m³)	(m ³)
19	75.62	20.50	23.37	8.09	0.18	9.41	13.96
23	67.26	18.23	25.16	8.09	0.22	11.39	13.77
30	56.60	15.35	27.62	8.09	0.29	14.85	12.77
60	34.64	9.39	33.81	8.09	0.58	29.71	4.10
120	20.34	5.52	39.71	8.09	1.17	59.42	0.00
180	14.73	3.99	43.13	8.09	1.75	89.13	0.00
nce: Soil Report for Wastewater Servicing by BOS Engineering & Environmental Services Inc. The percolation time (T) of the soil on-site is greater than 50 min/cm Max. Storage Volume					Max. Storage Volume (m³) =	13.96	

100 Year Design S	Storm Event						
		Inflow, Q _i	Volume In	Allowable Surface Outflow	Exfiltration	Total Volume Out	Difference/
Duration	Intensity "i"	2.78*C*i*A	Qt*t*60/1000	Q_{o}	Volume	Q _o *t*60/1000	Storage
(min.)	(mm/hr)	(I/s)	(m ³)	(I/s)	(m³)	(m³)	(m³)
19	131.48	35.65	40.64	14.26	0.18	16.44	24.20
23	117.50	31.86	43.96	14.26	0.22	19.90	24.06
30	99.36	26.94	48.49	14.26	0.29	25.96	22.53
60	60.87	16.50	59.41	14.26	0.58	51.91	7.49
120	35.32	9.58	68.94	14.26	1.17	103.83	0.00
180	25.28	6.85	74.02	14.26	1.75	155.74	0.00
Reference: Soil Report for Wastewater Serv	vicing by BOS Engineering & Environm	ental Services Inc. The percolation	on time (T) of the soil on-s	ite is greater than 50 min/cm		Max. Storage Volume (m³) =	24.20

Total Storage Available within infiltration trench (m³) =	24.26
Required 5 Year Storage (m³) =	13.96
Required 100 Year Storage (m³) =	24.20

	Drawdown time for soak-aw	ay pit
5 year storm events	23.94	hrs
100 year storm events	41.49	hrs

^{**}Time of concentration from Section 4.8.2 - Municipality of Middlesex Centre - Infrastructure Design Standards



1599 Adelaide St. N., Units 301 & 203 London, ON N5X 4E8 P: 519-471-6667

KITCHENER LOCATION

1415 Huron Rd., Unit 225 Kitchener, ON N2R 0L3 P: 519-725-8093

PLANNING • CIVIL • STRUCTURAL • MECHANICAL • ELECTRICAL **SWM Calculations**

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sbm@sbmltd.ca

DATE: March 18, 2021

JOB NO.: SBM-21-0716

Client: Brock Development Group
Project: Proposed Residential Development
Location: Medway Road, Ballymote, Ontario

CITY OF LONDON-3 CHICAGO RAINFALL DISTRIBUTION PARAMETERS*

Patura Pariod (vaars)		A,B,C Parameters	
Return Period (years)	Α	В	С
25mm	538.850	6.331	0.809
2	1290.000	8.500	0.860
5	1183.740	7.641	0.838
10	1574.382	9.025	0.860
25	2019.372	9.824	0.875
50	2270.665	9.984	0.876
100	2619.363	10.500	0.884
250	3048.220	10.030	0.888

Parks, open space and playgrounds = 0.2

Infrastructure Design Standards (IDS)

Standard Single Family Residential = 0.5

Runoff Coefficients from Section 4.8.3 of Municipality of Middlesex Centre

Post-Development Flow, $Q_r = 2.78 C^*i^A =$

A216 - LOT 16

Post-Development Flow, $Q_r = 2.78 \text{ °C*i*A} =$

PRE-DEVELOPMENT CONDITIONS (A216) POST-DEVELOPMENT CONDITIONS (A216)

PRE-DEVELOPMENT CONDITIONS (AZ	<u>.10)</u>		POST-DEVELOPIVIENT COINE	DITIONS (AZIO)	
	Area (m²)			Area (m²)	
Total Site Area:	1770.41		Total Site Area:	1770.41	
Runoff Coefficient C =	0.20		$C_{eq} = Sum(A*C)/Sum(A) =$	0.50	
5-Year Pre-Development Flows			100-Year Pre-Development Flow	'S	
C-value =	0.20		C-value =	0.20	
**Time of concentration t_c =	19	min	**Time of concentration t_c =	19	min
Intensity, i (@ t_c) =	74.60	mm/hr	Intensity, i (@ t_c) =	131.48	mm/hr
Pre-Development Flow, $Q_r = 2.78 \text{ °C*i*A} = $	7.34	I/s	Pre-Development Flow, Q _r = 2.78*C*i*A =	12.94	l/s
5-Year Post-Development Flows			100-Year Post-Development Flor	ws	
C-value =	0.50		C-value =	0.50	
**Time of concentration t_c =	19	min	**Time of concentration t_c =	19	min
Intensity, i (@ t_c) =	75.62	mm/hr	Intensity, i (@ t _c) =	153.12	mm/hr

^{**}Time of concentration from Section 4.8.2 - Municipality of Middlesex Centre - Infrastructure Design Standards

18.61

SOAK-AWAY PIT DETAILS

Stone Depth	1.0	m
Stone Width=	4.0	m
Stone Length=	21.0	m
19mm Clear Stone Void Ratio=	0.35	
Infiltration Rate=	1.33E-06	m/s
Storage (Total)=	29.4	m ³
Contact Area to Soil (Trench Side Walls Only)=	134.00	m^2
		-

Design Infiltration rate calculation

37.68

- coign minitiation rate carearation			
Coefficient of permeability (K)=	0.1	cm/sec	
Percolation time (T)=	50	mins/cm	
Infiltration rate (1/T)	12	mm/hr	
Safety correction factor	2.5		
Design Infiltration rate	1.33E-06	m/s	

(Table C3 TRCA)

(Conservative Assumption)

INFILTRATION RATE CALCULATIONS

A= I=		m ² Contact Area to So m/s
A*I	1.79E-04	m³/s

5 Year Design Sto	orm Event						
		Inflow, Q _i	Volume In	Allowable Surface Outflow	Exfiltration	Total Volume Out	Difference/
Duration	Intensity "i"	2.78*C*i*A	Qt*t*60/1000	Q_{o}	Volume	Q _o *t*60/1000	Storage
(min.)	(mm/hr)	(I/s)	(m³)	(I/s)	(m³)	(m³)	(m³)
19	75.62	18.61	21.21		0.20	0.20	21.01
23	67.26	16.55	22.84		0.25	0.25	22.59
30	56.60	13.93	25.07		0.32	0.32	24.75
60	34.64	8.52	30.69		0.64	0.64	30.04
120	20.34	5.01	36.05		1.29	1.29	34.76
180	14.73	3.62	39.15		1.93	1.93	37.22
Reference: Soil Report for Wastewater Servicing by BOS Engineering & Environmental Services Inc. The percolation time (T) of the soil on-site is greater than 50 min/cm Max						Max. Storage Volume (m³) =	37.22

100 Year Design	Storm Event						
		Inflow, Q _i	Volume In	Allowable Surface Outflow	Exfiltration	Total Volume Out	Difference/
Duration	Intensity "i"	2.78*C*i*A	Qt*t*60/1000	Q_{o}	Volume	Q _o *t*60/1000	Storage
(min.)	(mm/hr)	(I/s)	(m ³)	(I/s)	(m ³)	(m ³)	(m³)
19	131.48	32.36	36.89		0.20	0.20	36.68
23	117.50	28.92	39.90		0.25	0.25	39.66
30	99.36	24.45	44.01		0.32	0.32	43.69
60	60.87	14.98	53.92		0.64	0.64	53.28
120	35.32	8.69	62.58		1.29	1.29	61.29
180	25.28	6.22	67.19		1.93	1.93	65.26
eference: Soil Report for Wastewater Servicing by BOS Engineering & Environmental Services Inc. The percolation time (T) of the soil on-site is greater than 50 min/cm						Max. Storage Volume (m³) =	65.26

Total Storage Available within infiltration trench (m³) =	29.40	Soakaway pit drawdown time at 0.18L/s infiltration rate			Surface pond drawdown time a	at 0.18L/s infiltration rate	!
Required 5 Year Storage (m³) =	37.22	5 year storm events	57.87	hrs	_		
Required 100 Year Storage (m³) =	65.26	100 year storm events	101.46	hrs	All storm events	12.90	hrs

^{*} Intensity i=A/(t+B)^C (mm/hr)

^{*} Section 6.2 of Municipality of Middlesex Centre IDS, City of London a,b, & c design parameters are used.